


SHOP DRAWINGS		ABBREVIATIONS		GENERAL		LIST OF STRUCTURAL DRAWINGS																																												
<div>1. AS PART OF OUR CONSTRUCTION PHASE SERVICES, RJC WILL REVIEW SHOP DRAWINGS PERTAINING TO WORK SHOWN ON RJC'S DRAWINGS BY MEANS OF APPROPRIATE RATIONAL SAMPLING PROCEDURES AND COMMENT ON THE ACCURACY WITH WHICH THE CONTRACTOR PREPARED THE DRAWINGS.</div> <div>2. REVIEW OF SHOP DRAWINGS IS FOR THE SOLE PURPOSE OF ASCERTAINING CONFORMANCE WITH THE GENERAL DESIGN CONCEPT AND IS NOT AN APPROVAL OF THE DETAILED DESIGN INHERENT IN THE SHOP DRAWINGS. RESPONSIBILITY FOR WHICH SHALL REMAIN WITH THE CONTRACTOR SUBMITTING THEM. SUCH REVIEW SHALL NOT RELIEVE THE CONTRACTOR OF THEIR RESPONSIBILITY FOR ERRORS AND OMISSIONS IN THE SHOP DRAWINGS AND FOR MEETING ALL REQUIREMENTS OF THE CONTRACT DRAWINGS. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR INFORMATION PERTAINING TO THE FABRICATION PROCESS, TECHNIQUES FOR CONSTRUCTION AND INSTALLATION, AND FOR CO-ORDINATION OF THE WORK OF ALL SUB-TRADES.</div> <div>3. FOR SPECIFIC SHOP DRAWING SUBMITTAL REQUIREMENTS, SEE APPROPRIATE MATERIAL SECTIONS AND THE SPECIFICATIONS.</div> <div>4. SHOP DRAWINGS SHALL BE COMPLETE AND INCLUDE ANY REQUIRED SEALS FROM A PROFESSIONAL ENGINEER REGISTERED IN THE JURISDICTION WHERE THE PROJECT IS LOCATED PRIOR TO SUBMISSION.</div> <div>5. ALL SHOP DRAWINGS COMPRISING A REVISED SUBMISSION SHALL INDICATE THE REVISED CONTENT BY MEANS OF CLOUDING OR OTHER SUITABLE MARKINGS.</div>		<div>ACCOM. --- ACCOMMODATE</div> <div>AESS --- ARCHITECTURALLY EXPOSED STRUCTURAL STEEL</div> <div>AM --- FACTORED AXIAL FORCE</div> <div>ALT. --- ALTERNATE</div> <div>ALUM. --- ALUMINUM</div> <div>A.R. --- ANCHOR ROD</div> <div>ARCH. --- ARCHITECTURAL</div> <div>B.C.E. --- BOTTOM CHORD EXTENSION</div> <div>B.E.W. --- BOTTOM EACH WAY</div> <div>B.H.W. --- BOTTOM HORIZONTAL MEMBER</div> <div>B.L.W. --- BOTTOM LONG WAY</div> <div>BM. --- BEAM</div> <div>B.O.T. --- BOTTOM</div> <div>B.P.T. --- BUCKLING PREVENTION TIE</div> <div>B.S.W. --- BOTTOM SHORT WAY</div> <div>B.U.L. --- BOTTOM UPPER LAYER</div> <div>B.W. --- BOTH WAYS</div> <div>C.A. --- COLUMN ABOVE</div> <div>CANT. --- CANTILEVER</div> <div>C.B. --- COLUMN BELOW</div> <div>CBM. --- COUPLING BEAM</div> <div>C.C.P. --- COMPRESSION COUPLER</div> <div>CF. --- FACTORED AXIAL COMPRESSION FORCE</div> <div>C.I.P. --- CAST IN PLACE</div> <div>C.J. --- CONTROL JOINT</div> <div>CL. --- CENTER LINE</div> <div>CLR. --- CLEAR</div> <div>COL. --- COLUMN</div> <div>COMP. --- COMPRESSION</div> <div>CONC. --- CONCRETE</div> <div>CONT. --- CONTINUOUS</div> <div>C.P. --- COMPLETE PENETRATION</div> <div>C.S. --- COMPRESSION BRUCE</div> <div>CTRS. --- CENTERS</div> <div>C.W. --- COMPLETE WITH</div> <div>DBM. --- DIVIDER BEAM</div> <div>DET. --- DETAIL</div> <div>D.O. --- DEAD LOAD</div> <div>D.O. --- DO OVER: (DITTO)</div> <div>D.P. --- DEEP (E.C. DEPTH OF BEAM)</div> <div>D.S. --- DEPTH TO SUIT</div> <div>DWG. --- DRAWING</div> <div>DWLS. --- DOWELS</div> <div>E.A. --- EACH</div> <div>E.E. --- EACH END</div> <div>E.F. --- EACH FACE</div> <div>E.L. --- ELEVATION</div> <div>ELEV. --- ELEVATOR</div> <div>ELEC. --- ELECTRICAL</div> <div>EQ. --- EQUAL</div> <div>E.S. --- EACH SIDE</div> <div>E.W. --- EACH WAY</div> <div>EXIST. --- EXISTING</div> <div>EXP. JT. --- EXPANSION JOINT</div> <div>EXT. --- EXTERIOR</div> <div>F.D. --- FLOOR DRAIN</div> <div>F.F. --- FAR FACE</div> <div>F.S. --- FAR SIDE</div> <div>FTG. --- FOOTING</div> <div>GA. --- GAUGE</div> <div>GLV. --- GALVANIZED</div> <div>GL. --- GRID LINE</div> <div>GR. BM. --- GRADE BEAM</div> <div>G.W.B. --- GYPSUM WALL BOARD</div> <div>H. HORIZ. --- HORIZONTAL</div> <div>H.1.E. --- HOOK ONE END</div> <div>H.2.E. --- HOOK TWO ENDS</div> <div>H.W. --- HORIZONTAL AND VERTICAL</div> <div>H.D.G. --- HOT-DIP GALVANIZED</div> <div>M. --- FACTORED HORIZONTAL FORCE</div> <div>H.P. --- HIGH POINT</div> <div>H.S.C. --- HORIZONTALLY SLOTTED CONNECTION</div> <div>HT. --- HEIGHT</div> <div>I.F. --- INSIDE FACE</div> <div>INT. --- INTERIOR</div> <div>JT. --- JOINT</div> <div>L. --- LONG</div> <div>LL. --- LIVE LOAD</div> <div>LL.B. --- LONG LEGS BACK TO BACK</div> <div>LL.H. --- LONG LEG HORIZONTAL</div> <div>LL.V. --- LONG LEG VERTICAL</div> <div>L.P. --- LOW POINT</div> <div>L.S.H. --- LONG SIDE HORIZONTAL</div> <div>L.S.V. --- LONG SIDE VERTICAL</div>		<div>1. SECTION MARK SHOWN THIS  MEANS SECTION #4 ON DRAWING S-3.</div> <div>2. SEE ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS FOR SLEEVES, NAILERS, INSERTS, ETC. TO BE ENCASED IN CONCRETE.</div> <div>3. SEE ARCHITECTURAL DRAWINGS FOR FLOOR AND ROOF ELEVATIONS, RECESSES, DRAINAGE SLOPES, ETC.</div> <div>4. THE GENERAL CONTRACTOR SHALL REVIEW ALL THE DRAWINGS AND CHECK DIMENSIONS BEFORE CONSTRUCTION. REPORT DISCREPANCIES BETWEEN STRUCTURAL AND OTHER DISCIPLINE DRAWINGS FOR CLARIFICATION.</div> <div>5. CONCRETE WORK SHALL CONFORM TO CSA A23.1, CSA A23.2, CSA A23.3 AND REFERENCED DOCUMENTS.</div> <div>6. STRUCTURAL STEEL WORK SHALL CONFORM TO CSA S16 AND REFERENCED DOCUMENTS.</div> <div>7. FIRE RESISTANCE RATINGS DO NOT CARRY OVER FROM ARCHITECTURAL DRAWINGS AND SPECIFICATIONS FOR PRECISE DETERMINATION OF REQUIRED FIRE RESISTANCE RATINGS.</div> <div>8. DO NOT CUT OR DRILL ANY OPENINGS IN STRUCTURAL MEMBERS WITHOUT WRITTEN PERMISSION OF RJC.</div> <div>9. REFER TO ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND LANDSCAPE DRAWINGS FOR LOCATIONS, CONFIGURATIONS, EXTENT, AND SIZES OF ALL RISERS, UPSTAIRS, DOWNSTAIRS AND FOR OPENINGS THROUGH FLOORS AND WALLS FOR DUCTS, CONDUIT AND PIPING. PROVIDE FOR SAME.</div> <div>10. DEFINITIONS: A. <u>RJC</u>: READ JONES CHRISTOFFERSEN OR ITS REPRESENTATIVE. B. <u>REGISTERED STRUCTURAL ENGINEER</u>: A STRUCTURAL ENGINEER REGISTERED AND LICENSED TO PRACTICE BY THE PROFESSIONAL ENGINEERING ASSOCIATION HAVING JURISDICTION IN THE AREA WHERE THE STRUCTURE IS TO BE BUILT AND WHO IS RESPONSIBLE FOR THE DESIGN AND FIELD REVIEW OF: <ul style="list-style-type: none">STRUCTURAL ELEMENTS DESIGNED BY THE CONTRACTOR OR SUBCONTRACTORS, SUCH AS OPEN WEB STEEL JOISTS, PRECAST DOUBLE TEES, PRECAST PLANKS, STRUCTURAL STEEL CONNECTIONS, LIGHT WOOD FRAME ROOF TRUSSES, ETC.SECONDARY STRUCTURAL ELEMENTS AND NON-STRUCTURAL ELEMENTS, SUCH AS "NON-STRUCTURAL ELEMENTS" GENERAL NOTES. C. <u>CONTINUOUS</u>: FULL TENSION SLAB AND TENSION DEVELOPMENT LENGTH. D. <u>EMBEDMENT</u>: UNLESS NOTED OTHERWISE COMPRESSION EMBEDMENT MEANS A COMPRESSION DEVELOPMENT LENGTH AND TENSION EMBEDMENT MEANS A TENSION DEVELOPMENT LENGTH AS PER CANCSA-23 AND AS SHOWN ON THESE GENERAL NOTES DRAWINGS. E. <u>GENERAL CONTRACTOR</u>: FOR THE PURPOSES OF THESE DRAWINGS, THE USE OF THE TERM "CONTRACTOR" OR "GENERAL CONTRACTOR" SHALL REFER TO THE PRIME PERSON OR COMPANY RESPONSIBLE FOR CONSTRUCTION OF THE PROJECT AND THE COORDINATION OF TRADES AND SUBCONTRACTORS. THIS MAY BE THE GENERAL CONTRACTOR, OR A CONSTRUCTION MANAGER.</div>		<div>1.1 GENERAL NOTES</div> <div>1.2 GENERAL NOTES</div> <div>1.3 GENERAL NOTES</div> <div>1.4 GENERAL NOTES</div> <div>1.5 GENERAL NOTES</div> <div>1.6 GENERAL NOTES</div> <div>1.7 GENERAL NOTES</div> <div>1.8 GENERAL NOTES</div> <div>1.9 GENERAL NOTES</div> <div>1.10 GENERAL NOTES</div> <div>1.21 TYPICAL DETAILS</div> <div>1.22 TYPICAL DETAILS</div> <div>1.23 TYPICAL DETAILS</div> <div>1.24 TYPICAL DETAILS</div> <div>1.25 TYPICAL DETAILS</div> <div>1.26 TYPICAL DETAILS</div> <div>1.27 TYPICAL DETAILS</div> <div>1.28 TYPICAL DETAILS</div> <div>1.29 TYPICAL DETAILS</div> <div>1.30 TYPICAL DETAILS</div> <div>1.31 TYPICAL DETAILS</div> <div>1.32 MAIN FLOOR PLAN</div> <div>1.33 SECOND FLOOR PLAN</div> <div>1.34 SECOND FLOOR PLAN SHOWING THIRD FLOOR FRAMING OVER</div> <div>1.35 THIRD FLOOR PLAN SHOWING FOURTH FLOOR FRAMING OVER</div> <div>1.36 FOURTH FLOOR PLAN SHOWING FIFTH FLOOR FRAMING OVER</div>																																												
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<div>1. THE CONTRACTOR SHALL ENGAGE A SPECIALTY ENGINEER FOR THE DESIGN OF REQUIRED STRUCTURAL ELEMENTS AND REQUIRED STRUCTURAL CONNECTIONS NOT INDICATED IN THE DRAWINGS.</div> <div>2. STRUCTURAL COMPONENTS REQUIRING DESIGN COMPLETED BY THE CONTRACTOR'S SPECIALTY ENGINEER INCLUDE, BUT ARE NOT LIMITED TO, THE FOLLOWING: <ul style="list-style-type: none">A. COLD FORMED LIGHTWEIGHT STEEL FRAMINGB. MISCELLANEOUS STEELC. MORTAR, GROUT AND CONCRETE MIX DESIGNSD. GLUE-LAMINATED AND STRUCTURAL COMPOSITE LUMBER MEMBERS INCLUDING WOOD-TO-WOOD CONNECTIONS</div> <div>3. DESIGNS PROVIDED BY THE SPECIALTY ENGINEER SHALL CONSIDER STRENGTH, STABILITY, SERVICEABILITY AND INTEGRITY REQUIREMENTS UNDER GRAVITY AND SEISMIC LOADING AND THE DURABILITY FOR PREVAILING ENVIRONMENTAL AND EXPOSURE CONDITIONS. ALL DESIGNS SHALL BE IN ACCORDANCE WITH THE CURRENT EDITION OF APPLICABLE DESIGN CODES AND ALL OTHER DESIGN REQUIREMENTS INDICATED IN THE DRAWINGS AND SPECIFICATIONS. DESIGNS SHALL INCLUDE SUITABLE LETTERS OF ASSURANCE.</div> <div>4. DESIGN OF COMPONENTS AND CONNECTIONS THAT RELY ON SUPPORT BY THE PRIMARY STRUCTURE DESIGNED BY RJC OR COMPONENTS DESIGNED BY OTHER SPECIALTY ENGINEERS MUST CLEARLY INDICATE THE MEANS AND METHOD OF ATTACHMENT AND THE MAGNITUDE OF ALL FORCES (SPECIFIED AND FACTORED) THAT THE PRIMARY STRUCTURE MUST WITHSTAND. REVIEW BY THE STRUCTURAL ENGINEER OF RECORD MAY REQUIRE REVISION TO THE METHOD OF CONNECTION WITH REVISION BY THE SPECIALTY ENGINEER.</div> <div>5. SPECIALTY ENGINEERS ENGAGED BY THE CONTRACTOR SHALL BE REGISTERED AS PROFESSIONAL ENGINEERS IN THE PROVINCE OF SASKATCHEWAN AND ALL SUBMITTALS OR SHOP DRAWINGS PREPARED BY OR UNDER THE SUPERVISION OF THIS ENGINEER SHALL BE SIGNED. UNSEALED PROGRESS DOCUMENTS WILL BE REJECTED BY RJC WITHOUT REVIEW UNLESS PRIOR AGREEMENT IS OBTAINED.</div> <div>6. WHERE STRUCTURAL COMPONENTS OR CONNECTIONS DESIGNED BY THE SPECIALTY ENGINEER ARE TO BE FABRICATED IN A DIFFERENT JURISDICTION, THE SPECIALTY ENGINEER SHALL SUBMIT A SEALED LETTER CONFIRMING PROOF OF PROFESSIONAL REGISTRATION IN THE JURISDICTION OF FABRICATION.</div> <div>7. REFER TO THE DRAWINGS AND SPECIFICATIONS FOR OTHER REQUIREMENTS.</div>				<div>1. SPECIFIED UNIFORM LOADS - PSF</div> <table><thead><tr><th></th><th>LIVE LOAD</th><th>SUPERIMPOSED DEAD LOAD (S.D.L.)</th></tr></thead><tbody><tr><td>A. ROOF - BASED ON A GROUND SNOW LOAD OF 35.4</td><td>2.1</td><td>25</td></tr><tr><td>PLUS A RAIN LOAD OF 1.0 AND AN IMPORTANCE FACTOR OF $I_h = 1.0$ U.S. 9 SLS</td><td></td><td></td></tr><tr><td>B. RESIDENTIAL FLOORS</td><td>40</td><td>20</td></tr><tr><td>C. ROOF TERRACES</td><td>100</td><td>75</td></tr><tr><td>D. MECHANICAL ROOM</td><td>75</td><td>50</td></tr><tr><td>E. LOBBY LEVEL INTERIOR</td><td>100</td><td>50</td></tr><tr><td>F. PARKING LEVELS</td><td>50</td><td>5</td></tr><tr><td>G. STAIRS AND CORRIDORS</td><td>100</td><td>5</td></tr></tbody></table> <div>CONTRACTORS CONSTRUCTION LOADS MUST NOT EXCEED THE ABOVE. EXCEPT LOADS, DESIGN LOADS MAY ONLY BE APPLIED AFTER CONCRETE REACHES ITS DESIGN STRENGTH.</div> <div>SUPERIMPOSED DEAD LOADS (S.D.L.) ARE NON-STRUCTURE DEAD LOADS DUE TO ARCHITECTURAL TOPPINGS, FINISHES, PARTITIONS, ROOFING MATERIALS, PAVERS, SOIL, ETC.</div> <div>STRUCTURAL DEAD LOADS (D.L.) ARE DUE TO THE WEIGHT OF THE STRUCTURE ITSELF. THEY VARY WITH THE STRUCTURAL SYSTEM AND INCLUDE CONCRETE, TOPPINGS ON STEEL DECK.</div> <div>LIVE LOADS HAVE BEEN REDUCED FOLLOWING THE PROVISIONS SET FORTH IN THE NATIONAL BUILDING CODE OF CANADA 2020 FOR THE PURPOSE OF DESIGNING COLLUMNS, WALLS, TRANSFER BEAMS, TRANSFER SLABS, AND FOUNDATIONS.</div> <div>UNLESS NOTED OTHERWISE, SPECIFIED CONCENTRATED LOADS ARE:</div> <table><tbody><tr><td>A. ROOFS</td><td>0.3 KIPS</td></tr></tbody></table> <div>RAIN PONDING LOADS ON ROOFS ARE BASED ON ROOF SLOPES, PARAPET HEIGHTS AND SCUPPER LOADINGS SHOWN ON ARCHITECTURAL DRAWINGS. DEPTH OF PONDING ASSUMES THAT ALL ROOF DRAINS ARE ACCIDENTALLY PLUGGED FOR A MAXIMUM PERIOD OF 24 HOURS, UNLESS NOTED OTHERWISE.</div> <div>ONE DAY RAIN, (150) = 338"</div> <div>WIND UPLIFT LOADS ON STEEL OR WOOD ROOFS SHALL BE 20 PSF NET FACTORED UNLESS NOTED OTHERWISE.</div> <div>THIS PROJECT IS CLASSIFIED AS A NORMAL IMPORTANCE STRUCTURE.</div> <div>6. SEISMIC AND WIND DESIGN</div> <div>THE LATERAL SYSTEM FOR THIS PROJECT CONSISTS OF STEAR WALLS AND IS DESIGNED FOR THE FOLLOWING EARTHQUAKE FACTORS:</div> <table><tbody><tr><td>$S_a(0.2) = 0.094$</td><td>SITE CLASSIFICATION: SITE CLASS D</td></tr><tr><td>$S_a(0.5) = 0.064$</td><td>$I_e = 1.0$</td></tr><tr><td>$S_a(1.0) = 0.034$</td><td>$R_d = 3.0$</td></tr><tr><td>$S_a(2.0) = 0.015$</td><td>$R_o = 1.7$</td></tr><tr><td>$S_a(5.0) = 0.003$</td><td>$P_GA = 0.043$</td></tr><tr><td>$S_a(10.0) = 0.001$</td><td></td></tr></tbody></table> <div>HOURLY WIND PRESSURE, (150) = 9.58 PSF $I_e = 1.0$ U.S. 9.75 SLS</div>			LIVE LOAD	SUPERIMPOSED DEAD LOAD (S.D.L.)	A. ROOF - BASED ON A GROUND SNOW LOAD OF 35.4	2.1	25	PLUS A RAIN LOAD OF 1.0 AND AN IMPORTANCE FACTOR OF $I_h = 1.0$ U.S. 9 SLS			B. RESIDENTIAL FLOORS	40	20	C. ROOF TERRACES	100	75	D. MECHANICAL ROOM	75	50	E. LOBBY LEVEL INTERIOR	100	50	F. PARKING LEVELS	50	5	G. STAIRS AND CORRIDORS	100	5	A. ROOFS	0.3 KIPS	$S_a(0.2) = 0.094$	SITE CLASSIFICATION: SITE CLASS D	$S_a(0.5) = 0.064$	$I_e = 1.0$	$S_a(1.0) = 0.034$	$R_d = 3.0$	$S_a(2.0) = 0.015$	$R_o = 1.7$	$S_a(5.0) = 0.003$	$P_GA = 0.043$	$S_a(10.0) = 0.001$				<div>1. THIS SET OF DRAWINGS SHOWS THE COMPLETED PROJECT. THE DRAWINGS DO NOT SHOW COMPONENTS THAT MAY BE NECESSARY FOR CONSTRUCTION SAFETY. THE GENERAL CONTRACTOR IS RESPONSIBLE FOR SAFETY IN AND ABOUT THE JOB SITE DURING CONSTRUCTION, AND THE DESIGN AND ERECTION OF ALL TEMPORARY STRUCTURES, FORMWORK, FALSE WORK, SHORING, ETC. REQUIRED TO COMPLETE THE WORK.</div> <div>2. THE USE OF THESE DRAWINGS IS LIMITED TO THAT IDENTIFIED IN THE REVISIONS COLUMN. DO NOT CONSTRUCT FROM THESE DRAWINGS UNLESS MARKED "ISSUED FOR CONSTRUCTION" IN THE REVISIONS COLUMN. BY READ JONES CHRISTOFFERSEN LTD., THE DRAWINGS SHALL NOT BE USED FOR DESIGN, COSTING, OR TENDER UNLESS SO INDICATED IN THE REVISION COLUMN. PRICING OR COSTING DRAWINGS ARE NOT COMPLETE AND ANY PRICES BASED ON PRICING OR COSTING DRAWINGS MUST INCLUDE ALLOWANCES FOR THIS.</div> <div>3. THE INFORMATION ON THESE DRAWINGS SHALL NOT BE USED FOR ANY OTHER PROJECT OR WORKS. THE INFORMATION ON THESE DRAWINGS APPLIES SOLELY TO THIS PROJECT.</div> <div>4. GENERAL NOTES SHALL BE READ IN CONJUNCTION WITH THE TYPICAL DETAILS AND PROJECT SPECIFICATIONS.</div>	
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<div>1. READ JONES CHRISTOFFERSEN PROVIDES FIELD REVIEW ONLY FOR THE WORK SHOWN ON THESE STRUCTURAL DRAWINGS. THE REVIEW IS NOT A "FULL TIME" REVIEW BUT IS CONDUCTED WITH SUCH FREQUENCY AS RJC DEEMES APPROPRIATE TO OBSERVE THE WORK TO BE CONSTRUCTED AND TO ASCERTAIN THAT THE WORK IS IN GENERAL CONFORMANCE WITH THE PLANS AND SUPPORTING DOCUMENTS PREPARED BY READ JONES CHRISTOFFERSEN. FIELD REVIEW BY READ JONES CHRISTOFFERSEN IS NOT CARRIED OUT FOR THE CONTRACTOR'S BENEFIT. NOR DOES IT MAKE READ JONES CHRISTOFFERSEN GUARANTEE THE WORK OR THE CONTRACTOR'S WORK. IT REMAINS THE CONTRACTOR'S RESPONSIBILITY TO BUILD THE WORK IN CONFORMANCE WITH THE CONTRACT DOCUMENTS. RJC SHALL NOT BE RESPONSIBLE FOR THE ACTS OR OMISSIONS OF THE CONTRACTOR, SUB-CONTRACTOR, OR ANY OTHER PERSONS PERFORMING ANY OF THE WORK OR FOR THE FAILURE OF ANY OF THEM TO CARRY OUT THE WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.</div> <div>2. PROVIDE 24 HOURS ADVANCE NOTICE OF ANY REQUIRED FIELD REVIEW. FIELD REVIEWS SHALL BE SCHEDULED TO BE CARRIED OUT DURING NORMAL BUSINESS HOURS UNLESS SPECIAL ARRANGEMENTS ARE MADE WITH RJC.</div> <div>3. THE WORK TO BE REVIEWED SHALL BE GENERALLY COMPLETE.</div>				<div>1. THE STRUCTURAL DRAWINGS ARE BASED ON COORDINATED ARCHITECTURAL DRAWINGS.</div> <div>A. COORDINATION DATE: 2024-12-02</div> <div>DESIGN CODE</div> <div>1. THE COMPLETED BASE BUILDING STRUCTURE SHOWN ON THE STRUCTURAL DRAWINGS HAS BEEN DESIGNED IN SUBSTANTIAL ACCORDANCE WITH THE NATIONAL BUILDING CODE OF CANADA 2020.</div>																																														

1. THE CONTRACTOR SHALL ENGAGE A SPECIALTY ENGINEER FOR THE DESIGN OF REQUIRED STRUCTURAL ELEMENTS AND REQUIRED STRUCTURAL CONNECTIONS NOT INDICATED IN THE DRAWINGS.

2. STRUCTURAL COMPONENTS REQUIRING DESIGN COMPLETED BY THE CONTRACTOR'S SPECIALTY ENGINEER INCLUDE, BUT ARE NOT LIMITED TO, THE FOLLOWING:

A. COLD FORMED LIGHTWEIGHT STEEL FRAMING

B. MISCELLANEOUS STEEL

C. MORTAR, GROUT AND CONCRETE MIX DESIGNS

D. GLUE-LAMINATED AND STRUCTURAL COMPOSITE LUMBER MEMBERS INCLUDING WOOD-TO-WOOD CONNECTIONS

3. DESIGNS PRODUCED BY THE SPECIALTY ENGINEER SHALL CONSIDER STRENGTH, STABILITY, SERVICEABILITY AND INTEGRITY REQUIREMENTS UNDER GRAVITY AND SEISMIC LOADING AND THE DURABILITY FOR PREVAILING ENVIRONMENTAL AND EXPOSURE CONDITIONS. ALL DESIGNS SHALL BE IN ACCORDANCE WITH THE CURRENT EDITION OF APPLICABLE DESIGN CODES AND ALL OTHER DESIGN REQUIREMENTS INDICATED IN THE DRAWINGS AND SPECIFICATIONS. DESIGNS SHALL INCLUDE SUITABLE LETTERS OF ASSURANCE.

4. DESIGN OF COMPONENTS AND CONNECTIONS THAT RELY ON SUPPORT BY THE PRIMARY STRUCTURE DESIGNED BY RJC OR COMPONENTS DESIGNED BY OTHER SPECIALTY ENGINEERS MUST CLEARLY INDICATE THE MEANS AND METHOD OF ATTACHMENT AND THE MAGNITUDE OF ALL FORCES (SPECIFIED AND FACTORED) THAT THE PRIMARY STRUCTURE MUST WITHSTAND. REVIEW BY THE STRUCTURAL ENGINEER OF RECORD MAY REQUIRE REVISION TO THE METHOD OF CONNECTION WITH REDESIGN BY THE SPECIALTY ENGINEER.

5. SPECIALTY ENGINEERS ENGAGED BY THE CONTRACTOR SHALL BE REGISTERED AS PROFESSIONAL ENGINEERS IN THE PROVINCE OF SASKATCHEWAN AND ALL SUBMITTALS OR SHOP DRAWINGS PREPARED BY OR UNDER THE SUPERVISION OF THIS ENGINEER SHALL BE SIGNED. UNSEALED PROGRESS DOCUMENTS WILL BE REJECTED BY RJC WITHOUT REVIEW UNLESS PRIOR AGREEMENT IS MADE.

6. WHERE STRUCTURAL COMPONENTS OR CONNECTIONS DESIGNED BY THE SPECIALTY ENGINEER ARE TO BE FABRICATED IN A DIFFERENT JURISDICTION, THE SPECIALTY ENGINEER SHALL SUBMIT A SEALED LETTER CONFIRMING PROOF OF PROFESSIONAL REGISTRATION IN THE JURISDICTION OF FABRICATION.

7. REFER TO THE DRAWINGS AND SPECIFICATIONS FOR OTHER REQUIREMENTS.



Read Jones Christoffersen Ltd.
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220 20th Street West, Suite 112
Saskatoon, SK S7M 0V9 Canada
tel 306-888-2570

1	ISSUED FOR 50% REVIEW	2024.12.20	KML
No.	Revision	Date	By

Drawings Notes

All drawings, plans, models, designs, specifications and other documents prepared by Read Jones Christoffersen Ltd. ("RJC") and used in connection with this project are instruments of service for the work shown in them. The "Work" and its such as and remain the property of RJC. Where the Work is executed or not, and RJC reserves the copyright in them and in the Work executed from them, and they shall not be used for any work or project.

These drawings are "design drawings" only. They may not be suitable for use as shop drawings. Use of these drawings as base drawings for "shop drawings" is not permitted unless written permission containing certain conditions and limitations is obtained from RJC. The work "as constructed" may vary from what is shown on these drawings.

Use of these drawings is limited to that identified in the Issues/Revision column. Do not construct from these drawings unless marked "Issued for Construction" by RJC in the Issues/Revision column, and then only for the parts noted. The drawings shall not be used for "pricing", "bidding" or "tender" unless so indicated in the Issues/Revision column. "Pricing" or "bidding" drawings are not complete and any prices based on such drawings must allow for this.

Seal

NOT FOR CONSTRUCTION

Project Name
PROPOSED COLLEGE DRIVE APARTMENTS

1202 COLLEGE DRIVE SASKATOON, SK

Sheet Title
GENERAL NOTES

Drawn By	KML	Scale	As Indicated
Designed By	PWM	Date	2024.12.20
RJC Project Number	SAS.138936.0001		

Sheet Number
S 1.1
Revision
1
2024-12-20 18:56:20



Creative Thinking
Practical Results

Read Jones Christoffersen Ltd.
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tel 306-808-2550

CONCRETE - STRENGTH AND EXPOSURE				GRADE BEAM REINFORCEMENT		HELICAL SCREW PILE FOUNDATIONS		NON-STRUCTURAL ELEMENTS																																							
GENERAL (AREAS NOT INCLUDING PARKING)				<div><div><div><div><div></div><div>2-20M TOP</div></div><div><div>15M @ 8" HORIZ. EACH FACE</div><div>10M @ 8" CLOSED STIRRUPS</div><div>2-20M BOTTOM</div></div></div></div><div><div><div>12" MAX. TYP.</div><div>OUTER CLOSED STIRRUP</div><div>20M @ 12" TOP</div><div>15M @ 8" HORIZ. EACH FACE</div><div>10M @ 12" STIRRUPS</div><div>2-20M @ 12" BOTTOM</div><div>OPEN STIRRUPS OR HAIR PIN BARS WITH 135° HOOKS OR BAR TERMINATORS EACH END</div></div></div></div>		<div><div><div>1. PILING SHALL BE PERFORMED BY FIRMS SPECIALIZING IN THE DESIGN AND INSTALLATION OF HELICAL SCREW PILE FOUNDATIONS. PROVIDE CERTIFICATION DOCUMENTS TO THE OWNER OR THEIR REPRESENTATIVE.</div><div>2. PILING AND PILE DESIGN SHALL CONFORM TO THE NATIONAL BUILDING CODE OF CANADA 2020 AND TO COMMENTARY K OF THE USER'S GUIDE- NBC 2020 STRUCTURAL COMMENTARIES PART 4 - DIVISION B7. REFER TO PLANS FOR FACTORED PILE DESIGN LOADING REQUIREMENTS.</div><div>3. UNDER SPECIFIED LOADS, HELICAL SCREW PILES SHALL NOT SETTLE MORE THAN 5mm. THIS REQUIREMENT SHALL BE CONFIRMED BY PILE TESTING AS OUTLINED BELOW: A. LOAD TESTS SHALL BE PERFORMED TO VERIFY THE SUITABILITY AND CAPACITY OF THE PROPOSED SCREW PILE AND THE PROPOSED INSTALLATION PROCEDURES PRIOR TO INSTALLATION OF PRODUCTION PILES. TESTING SHALL CONFORM TO ASTM STANDARD D1143.</div><div>4. A MINIMUM OF ONE SACRIFICIAL TEST PILE WITH REACTION ANCHORS SHALL BE CONSTRUCTED PRIOR TO THE START OF WORK AT LOCATION AS DIRECTED BY THE OWNER.</div><div>5. TESTING SHALL BE USED TO VERIFY THE SCREW PILE DESIGN AND CONFIRM THAT PILE SETTLEMENT UNDER SPECIFIED LOADS DOES NOT EXCEED 5 mm.</div><div>6. PROVIDE THE OWNER COPIES OF FIELD TESTING REPORTS WITHIN 24 HOURS AFTER THE COMPLETION OF THE LOAD TEST. THIS WRITTEN DOCUMENT WILL EITHER CONFIRM THE LOAD CAPACITY AS REQUIRED ON THE DRAWINGS OR PROPOSE CHANGES BASED UPON THE RESULTS OF THE LOAD TEST.</div><div>7. ALL MATERIALS FORMING PART OF THE SCREW PILE ASSEMBLY SHALL STRICTLY ADHERE TO MANUFACTURER'S REQUIREMENTS.</div><div>8. HELICAL BEARING PLATE, CENTRAL SHAFT (HEAD AND EXTENSION SECTIONS) AND ALL OTHER COMPONENTS IN CONTACT WITH THE SOIL SHALL BE HOT-DIPPED GALVANIZED.</div><div>9. SCREW PILES SHALL BE INSTALLED IN STRICT ACCORDANCE WITH THE MANUFACTURER'S INSTALLATION PROCEDURES.</div><div>10. SCREW PILES SHALL BE DRIVEN VERTICALLY WITH A VARIATION OF NOT MORE THAN 10 mm PER METRE. ALL HEADS SHALL BE WITHIN 75 mm OF THEIR SHOWN LOCATIONS. MINIMUM INSTALLATION TORQUE AND MINIMUM OVERALL LENGTH CRITERIA AS SHOWN ON THE WORKING DRAWINGS SHALL BE SATISFIED PRIOR TO TERMINATING THE SCREW PILE. IF THE SCREW PILE IS REFUSED OR DEFLECTED BY A SUBSURFACE OBSTRUCTION, THE INSTALLATION SHALL BE TERMINATED AND THE PILE REMOVED. THE OBSTRUCTION SHALL BE REMOVED AND THE SCREW PILE RE-INSTALLED. IF THE OBSTRUCTION CANNOT BE REMOVED, THE SCREW PILE SHALL BE INSTALLED AT AN ADJACENT LOCATION SUBJECT TO REVIEW AND ACCEPTANCE BY THE OWNER.</div><div>11. SCREW PILE CUT-OFF SHALL BE TRUE AND LEVEL WITHIN 25 mm OF THE SPECIFIED CUT-OFF ELEVATIONS.</div><div>12. SUBMIT SHOP DRAWINGS FOR ALL SCREW PILE COMPONENTS, INCLUDING CASING COMPONENTS AND PILE TOP ATTACHMENT. THIS INCLUDES SCREW PILE LEAD AND EXTENSION SECTION IDENTIFICATION. ALL SUBMITTALS SHALL BE SIGNED AND SEALED BY A REGISTERED PROFESSIONAL ENGINEER LICENSED IN THE PROVINCE OF ALBERTA.</div><div>13. THE CONTRACTOR SHALL SUBMIT RECORD DRAWINGS SHOWING THE ACCURATE LOCATIONS AND DETAILS OF AS-BUILT PILES AND PROVIDE WRITTEN CONFIRMATION THAT AS-BUILT PILES HAVE BEEN INSTALLED IN CONFORMANCE WITH ENGINEERED SHOP DRAWINGS.</div></div></div>		<div><div><div>1. "NON-STRUCTURAL" OR "SECONDARY STRUCTURAL" ELEMENTS ARE NOT PART OF THE STRUCTURAL DESIGN SHOWN ON THESE DRAWINGS. SUCH ELEMENTS ARE DESIGNED, DETAILED AND REVIEWED IN THE FIELD BY OTHERS. THEY APPEAR ON DRAWINGS OTHER THAN THESE DRAWINGS OF READ JONES CHRISTOFFERSEN LTD. WHERE STRUCTURAL ENGINEERING RESPONSIBILITY IS REQUIRED FOR THESE ELEMENTS, THIS SHALL BE PROVIDED BY SPECIALTY STRUCTURAL ENGINEERS, WHO SHALL PREPARE ALL SUBMITTALS UNDER THEIR SEAL AND SIGNATURE AND ALSO PROVIDE ANY LETTERS REQUIRED BY BUILDING PERMIT AUTHORITIES.</div><div>2. EXAMPLES OF NON-STRUCTURAL ELEMENTS INCLUDE, BUT ARE NOT LIMITED TO: A. ARCHITECTURAL COMPONENTS SUCH AS GUARDRAILS, HANDRAILS, FLAG POSTS, CANOPIES, CEILINGS, MILLWORK, ETC. B. LANDSCAPE ELEMENTS SUCH AS BENCHES, LIGHT POSTS, PLANTERS, PAVERS, SUPPORT PEDESTALS, ETC. C. CLADDING, GLAZING, WINDOW MULLIONS, INTERIOR STUD WALLS AND EXTERIOR STUD WALLS. D. ARCHITECTURAL PRECAST, PRECAST CLADDING. E. SKYLIGHTS. F. MECHANICAL AND ELECTRICAL EQUIPMENT, COMPONENTS, AND THEIR ATTACHMENT DETAILS. G. WINDOW WASHING EQUIPMENT AND ITS ATTACHMENTS. H. FALL PROTECTION AND FALL ARREST SYSTEMS AND THEIR ATTACHMENTS. I. ESCALATORS, ELEVATORS, AND CONVEYING SYSTEMS. J. GLASS BLOCK AND ITS ATTACHMENTS. K. BRICK OR BLOCK VENEERS AND THEIR ATTACHMENTS. L. DESIGN AND FIELD REVIEW OF SEISMIC RESTRAINT FOR SECONDARY STRUCTURAL ELEMENTS AND OPERATIONAL AND FUNCTIONAL COMPONENTS INCLUDING MECHANICAL AND ELECTRICAL EQUIPMENT. M. NON-STRUCTURAL CONCRETE TOPPING. N. DESIGN AND FIELD REVIEW OF NON-LOAD BEARING MASONRY.</div><div>3. DESIGNS PRODUCED BY THE SPECIALTY ENGINEER SHALL CONSIDER STRENGTH, STABILITY, SERVICEABILITY AND INTEGRITY REQUIREMENTS UNDER GRAVITY AND SEISMIC LOADING IN ACCORDANCE WITH THE CURRENT EDITION OF APPLICABLE DESIGN CODES AND ALL OTHER DESIGN REQUIREMENTS INDICATED IN THE DRAWINGS AND SPECIFICATIONS.</div><div>4. CONTRACTOR SHALL COORDINATE THE DESIGN OF ALL NON-STRUCTURAL ELEMENTS DESIGNED BY ONE OR MORE SPECIALTY ENGINEERS AND CONNECTING TO ELEMENTS DESIGNED BY OTHER SPECIALTY ENGINEERS TO ENSURE THE STRENGTH, STABILITY, SERVICEABILITY AND INTEGRITY OF THE FINAL CONSTRUCTION.</div><div>5. SHOP DRAWINGS FOR NON-STRUCTURAL ELEMENTS WHICH MAY AFFECT THE PRIMARY STRUCTURAL SYSTEM SHALL BE SUBMITTED TO READ JONES CHRISTOFFERSEN LTD. INDICATE CLEARLY THE METHOD OF ATTACHMENT AND MAGNITUDE OF ALL FORCES (SPECIFIED AND FACTORED) THAT THE STRUCTURE MUST WITHSTAND. THESE DRAWINGS WILL BE REVIEWED ONLY FOR THE EFFECT OF THE ELEMENT ON THE PRIMARY STRUCTURAL SYSTEM.</div></div></div>																																							
<table><tr><th>ELEMENT</th><th>COMPRESSIVE STRENGTH (MPa) 28 DAY U.N.O.</th><th>EXPOSURE CLASS</th><th>COMMENTS</th></tr><tr><td>SLAB ON GRADE (INTERIOR)</td><td>25 MPa</td><td>N</td><td></td></tr><tr><td>SLAB ON GRADE (EXTERIOR)</td><td>32 MPa</td><td>C-2</td><td></td></tr><tr><td>RETAINING WALLS / FOUNDATION WALLS</td><td>25 MPa</td><td>F-2</td><td>19mm AGGREGATE</td></tr><tr><td>SHEAR WALLS</td><td>SEE SCHEDULE (50 DAY)</td><td>NF-2</td><td></td></tr><tr><td>OTHER WALLS</td><td>25 MPa</td><td>NF-2</td><td></td></tr><tr><td>COLUMNS</td><td>SEE SCHEDULE (50 DAY)</td><td>NF-2</td><td></td></tr><tr><td>MECHANICAL HOUSEKEEPING PADS</td><td>20 MPa</td><td>N</td><td></td></tr><tr><td>SLABS AND BEAMS</td><td>25 MPa (28 DAY)</td><td>N</td><td></td></tr><tr><td>EXTERIOR EXPOSED BALCONIES AND OVERBAYS</td><td>35 MPa</td><td>F-1</td><td>SEE PLANS AND ARCH. DWGS.</td></tr></table>				ELEMENT	COMPRESSIVE STRENGTH (MPa) 28 DAY U.N.O.	EXPOSURE CLASS	COMMENTS	SLAB ON GRADE (INTERIOR)	25 MPa	N		SLAB ON GRADE (EXTERIOR)	32 MPa	C-2		RETAINING WALLS / FOUNDATION WALLS	25 MPa	F-2	19mm AGGREGATE	SHEAR WALLS	SEE SCHEDULE (50 DAY)	NF-2		OTHER WALLS	25 MPa	NF-2		COLUMNS	SEE SCHEDULE (50 DAY)	NF-2		MECHANICAL HOUSEKEEPING PADS	20 MPa	N		SLABS AND BEAMS	25 MPa (28 DAY)	N		EXTERIOR EXPOSED BALCONIES AND OVERBAYS	35 MPa	F-1	SEE PLANS AND ARCH. DWGS.	<div><div><div>2. HOOK HORIZONTAL BEAM REINFORCING OR PROVIDE CORNER BARS AT ALL BEAM INTERSECTIONS AND CORNERS PER DETAILS CF130 & CF131.</div><div>3. UNLESS NOTED OTHERWISE BEAM REINFORCEMENT SHALL BE CONTINUOUS.</div><div>4. TOP REINFORCEMENT SHALL BE SPLICED AT MIDSPAN AND SHALL HAVE LAPS OF 36" HOOK TOP REINFORCEMENT AT END SUPPORTS AND PROVIDE ADDITIONAL TOP BARS WHERE REINFORCEMENT IS INTERRUPTED BY RECESSED COLUMN OR BASEPLATE DETAILS.</div><div>5. BOTTOM REINFORCEMENT SHALL BE SPLICED AT PILE, OR FOUNDATION LOCATIONS AND SHALL HAVE LAPS OF 30".</div><div>6. ALL REINFORCEMENT TO BE CONTINUOUS THROUGH PILE CAPS, FOUNDATIONS, OR INTERSECTING GRADE BEAMS.</div><div>7. UNLESS OTHERWISE NOTED MAXIMUM LENGTH OF POUR SHALL NOT EXCEED 100' / LOCATIONS AND DETAILS OF CONSTRUCTION JOINTS SHALL BE SUBMITTED TO THE CONSULTANT FOR REVIEW PRIOR TO CONSTRUCTION.</div><div>8. REFER TO STRUCTURAL DRAWINGS FOR DETAILS OF GRADE BEAM CONSTRUCTION JOINTS AND CONTROL JOINTS PER DETAIL CF132</div><div>9. TOP OF GRADE BEAM TO BE FLUSH WITH TOP OF END SUPPORT U.N.O.</div></div></div>			
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NOTES:				CONCRETE - GENERAL																																											
1. WHERE EXPOSURE CLASS LISTED AS NF-1-F-2:				1. UNLESS NOTED OTHERWISE, ALL CONCRETE IS TO BE CAST-IN-PLACE.																																											
A. USE N2 EXPOSURE FOR INTERIOR CONCRETE LOCATED WITHIN AN INSULATED BUILDING ENVELOPE (E.G. DRY AND NOT SUBJECT TO FREEZING AND THAWING).				2. THE USE OF SHOTCRETE REQUIRES APPROVAL BY THE STRUCTURAL ENGINEER. ANY COSTS ASSOCIATED WITH REDESIGN, CHANGES TO THE CONTRACT DOCUMENTS AND ANY ADDITIONAL TESTING AND CONTRACT ADMINISTRATION COSTS TO ACCOMMODATE SHOTCRETE IS TO BE PAID FOR BY THE CONTRACTOR.																																											
B. USE F-1 EXPOSURE FOR HORIZONTAL AND SLOPED CONCRETE MEMBERS EXTERIOR TO THE BUILDING INSULATION AND NOT PROTECTED BY A MEMBRANE AND DRIP EDGE (E.G. WET AND SUBJECT TO FREEZING AND THAWING).				3. PORTLAND CEMENT SHALL BE TYPE GU OR GUL UNLESS NOTED OTHERWISE. ALL CONCRETE MIX SUBMITTALS MUST CLEARLY INDICATE THE SPECIFIC CEMENT TYPE TO BE UTILIZED, OR THE PROPORTIONS WHEN MULTIPLE CEMENT TYPES ARE UTILIZED IN THE SAME MIX.																																											
C. USE F-2 EXPOSURE FOR HORIZONTAL AND SLOPED CONCRETE MEMBERS EXTERIOR TO THE BUILDING INSULATION AND PROTECTED BY A MEMBRANE AND DRIP EDGE (E.G. DRY AND SUBJECT TO FREEZING AND THAWING).				4. CEMENT TYPE AND SUPPLEMENTARY CEMENTING MATERIALS FOR EXPOSURE CLASSES S-1, S-2, AND S-3 SHALL BE AS OUTLINED IN CSA A23.1.																																											
D. USE F-2 FOR VERTICAL CONCRETE MEMBERS EXTERIOR TO THE BUILDING INSULATION.				5. CONCRETE SHALL HAVE A UNIT WEIGHT OF 23kN/m ³ (145kN/PCF) UNLESS NOTED OTHERWISE.																																											
2. CONCRETE STRENGTH AND EXPOSURE CLASS OF STAIRS AND RAMPS SHALL MEET THE MOST STRINGENT CRITERIA OF THE ADJOINING SLABS AND BEAMS UNLESS NOTED OTHERWISE.				6. THE CONCRETE PROPERTIES USED IN DESIGN ARE BASED ON A NOMINAL COARSE AGGREGATE SIZE OF 20 mm (3/4") ACCORDING TO TABLE 11 OF CSA A23.1. UNLESS NOTED OTHERWISE, ALL LOCATIONS PROPOSED BY THE CONTRACTOR FOR USE OF CONCRETE MIX DESIGNS WITH A NOMINAL COARSE AGGREGATE SIZE DIFFERENT THAN 20 mm (3/4") SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER FOR REVIEW AND APPROVAL. ANY INCREASE IN REQUIRED CONCRETE STRENGTH OR INCREASE IN QUANTITY OF REINFORCEMENT DUE TO PROPOSED USE OF CONCRETE MIX WITH DIFFERENT NOMINAL COARSE AGGREGATE SIZE TO BE PAID FOR BY THE CONTRACTOR.																																											
PARKING AREAS				7. RECYCLED AGGREGATE IS NOT TO BE USED WITHOUT WRITTEN APPROVAL BY THE STRUCTURAL ENGINEER.																																											
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				9. MAXIMUM WATER/CEMENT RATIO AND AIR CONTENT TO MEET THE REQUIREMENTS FOR THE EXPOSURE CLASS AS OUTLINED IN CSA A23.1. REQUIRED ADMIXTURES TO BE USED: P-1, P-2, C-1, C-2 AND C-3. C-XL SHALL BE BASED ON CONCRETE EXPOSED TO FREEZE-THAW CYCLES UNLESS NOTED OTHERWISE.																																											
				10. CHLORIDE ION PENETRABILITY FOR EXPOSURE CLASS C-1 AND C-XL SHALL MEET THE REQUIREMENTS OF CSA A23.1.																																											
CONCRETE - SUPPLY, TESTING AND SUBMITTALS						CONCRETE - FINISHING AND ADMIXTURES																																									
1. CONCRETE IS SPECIFIED AS PER THE "PERFORMANCE" ALTERNATE AS OUTLINED IN CSA A23.1.						1. CURING OF CONCRETE TO MEET THE REQUIREMENTS FOR THE EXPOSURE CLASS AS OUTLINED IN CSA A23.1. CURING COMPOUNDS ARE NOT PERMITTED FOR SUSPENDED PARKING SLABS OR EXPOSURE CLASS C-XL CONCRETE. PARKING SLABS AND REINFORCED SLAB ON GRADES IN PARKING AREAS ARE TO BE CURED FOR MINIMUM 7 DAYS.																																									
2. THE GENERAL CONTRACTOR IS RESPONSIBLE FOR WORKING WITH THE CONCRETE SUPPLIER TO ENSURE THAT THE PLASTIC AND HARDENED MIX PROPERTIES MEET SITE REQUIREMENTS FOR PLACING, FINISHING, AND THE OWNER'S SPECIFIED PERFORMANCE REQUIREMENTS. THE GENERAL CONTRACTOR SHALL MEET THE DOCUMENTATION AND QUALITY CONTROL REQUIREMENTS OUTLINED UNDER THE "PERFORMANCE" ALTERNATE OF CSA A23.1.						2. CORROSION INHIBITORS ARE TO BE USED IN CONCRETE IN AREAS NOTED ON THE STRUCTURAL DRAWINGS, AS WELL AS IN STAIRS AND STAIR LANDINGS WITH PARADES. USE 10LW OF "DOL" 5% BY GRADE. CONSTRUCTION PRODUCTS OR MASTERFLEX IC 30 BY BASF CONSTRUCTION CHEMICALS. ALTERNATIVELY, USE C-XL CONCRETE WITH CURING TYPE 3 (EXTENDED) PER CSA A23.1.																																									
3. THE SUPPLIER SHALL MEET ALL CERTIFICATION AND DOCUMENTATION REQUIREMENTS AS OUTLINED UNDER THE "PERFORMANCE" ALTERNATIVE OF CSA A23.1.						3. ALL BOTTOM EDGES OF EXPOSED SLABS AND BEAMS, AS WELL AS EDGES OF WALLS AND COLUMNS, TO BE CHAMFERED 3/4" X 3/4". ALL TOP EDGES OF EXPOSED SLABS, BEAMS, UPSTANDS AND STAIRS TO BE TOOLED UNLESS NOTED OTHERWISE. PROVIDE FINISHING DETAILS, DRAWINGS AND SPECIFICATIONS FOR OTHER FINISH REQUIREMENTS.																																									
4. SUBMIT A MIX DESIGN REVIEW LETTER SIGNED AND SEALED BY A PROFESSIONAL ENGINEER CONFIRMING THAT THE PROPOSED MIX DESIGNS WILL ACHIEVE THE REQUIRED STRENGTH, DURABILITY, AND PERFORMANCE REQUIREMENTS INDICATED UNDER SUPPLIER RESPONSIBILITY - ITEM (g) OF TABLE 5 (ALTERNATIVE 1) OF CSA A23.1.						4. NO CALCIUM CHLORIDE IS PERMITTED, IN ANY FORM, IN ANY CONCRETE MIX WITHOUT THE EXPRESS WRITTEN CONSENT OF READ JONES CHRISTOFFERSEN LTD.																																									
5. AT THE REQUEST OF THE OWNER, THE SUPPLIER WILL FURNISH TEST DATA RESULTS (LESS THAN 3 MONTHS OLD) FOR EACH PROPOSED MIX DESIGN DEMONSTRATING THAT THEY MEET THE STRENGTH, DURABILITY, AND SHRINKAGE REQUIREMENTS SPECIFIED.						5. CURING AND PROTECTION OF CONCRETE FOR HOT, COLD OR DRY WEATHER IS TO BE AS PER CSA A23.1 AS A MINIMUM. SEE ALSO "CONCRETE COLD WEATHER REQUIREMENTS" IN THE STRUCTURAL DRAWINGS.																																									
6. THE CONCRETE SUPPLIER SHALL BE CERTIFIED BY THE READY MIXED CONCRETE ASSOCIATION OF SASKATCHEWAN.																																															

Project Name
**PROPOSED COLLEGE
DRIVE APARTMENTS**

1202 COLLEGE DRIVE
SASKATOON, SK

Sheet Title
GENERAL NOTES

Drawn By **PWM** Scale **As Indicated**
Designed By **KML** Date **2024.12.20**
RJC Project Number **SAS.138936.0001**

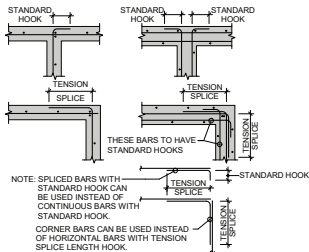
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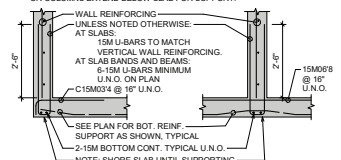
WALLS - CORNER DETAILS

1. DETAILS OF HORIZONTAL REINFORCEMENT AT CORNERS (SEE ALSO ZONE REINFORCING DETAILS).

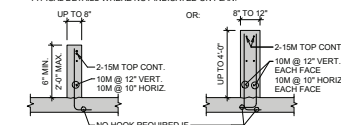


WALLS - MISC. DETAILS

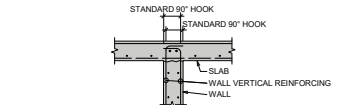
1. UNLESS NOTED OTHERWISE, PROVIDE U-BARS AS SHOWN WHERE NO WALLS OR COLUMNS EXTEND BELOW SLAB FOR SUPPORT.



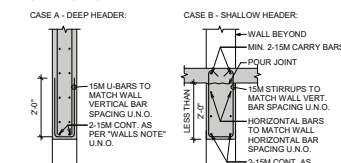
2. SEE ARCHITECTURAL DRAWINGS FOR EXTENT, THICKNESS, AND LOCATION OF CONCRETE UPSTAND WALLS, PLANTER WALLS AND CURBS. BELOW ARE TYPICAL DETAILS WHERE NOT INDICATED ON PLAN:



3. TOP OF WALL TRANSITION UNLESS NOTED OTHERWISE:

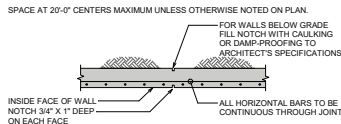


4. NOMINAL U-BARS / STIRRUPS ABOVE WALL OPENINGS UP TO 4'-0" WIDE UNLESS NOTED OTHERWISE:



WALL CONTROL JOINT

UNLESS NOTED OTHERWISE FOR EXTERIOR WALLS BELOW GRADE AND EXTERIOR WALLS EXPOSED TO WEATHER ABOVE GRADE.



EMBEDMENT / DEVELOPMENT LENGTHS AND SPICE LENGTHS

CASE 2 TENSION EMBEDMENT AND SPICE CONDITIONS
TENSION EMBEDMENT AND SPICE LENGTHS CONFORMING TO CSA A23.3 TABLE 12.1 (0.8A_sA_uA_u / f_t) ARE TO BE AS PER THE FOLLOWING TABLE FOR MEMBERS NOT SATISFYING CASE 1 CONDITIONS AS SET OUT ABOVE. FOR EXAMPLE:
A. ONE WAY SLAB TOP BARS (SEE TOP BAR NOTE).
B. BARS (EXCLUDING THE SPICE) SPACED CLOSER TOGETHER THAN 2 BAR DIAMETERS IN SAME LAYER AND BETWEEN LAYERS.
C. STIRRUPS IN BEAMS AND GIRDERS.
D. SEE ALSO NOTES ON TOP BARS.

CONCRETE STRENGTH	FUNCTION	REBAR DESIGNATION (GRADE 400 LENGTHS)					
		10M	15M	20M	25M	30M	35M
20 MPa	EMBEDMENT	17"	26"	34"	53"	64"	74"
	(SPICE)	(22")	(33")	(44")	(69")	(83")	(97")
25 MPa	EMBEDMENT	16"	23"	31"	48"	57"	67"
	(SPICE)	(20")	(27")	(37")	(52")	(64")	(74")
30 MPa	EMBEDMENT	14"	21"	28"	44"	52"	61"
	(SPICE)	(18")	(27")	(36")	(57")	(68")	(79")
35 MPa & GREATER	EMBEDMENT	13"	20"	26"	40"	48"	56"
	(SPICE)	(17")	(25")	(34")	(52")	(63")	(73")

NOTES:

1. TOP BAR VALUES ARE 1.3 TIMES THE ABOVE LENGTHS. TOP BAR APPLIES TO HORIZONTAL REINFORCEMENT CAST WITH 12" OR MORE OF CONCRETE BELOW THE BAR.
2. INCREASE THESE TABLE LENGTHS BY 1.15 TIMES WHEN SPACING BETWEEN LAYERS OF REBAR IS 1.0m.
3. WHERE A TENSION SPICE IS SPECIFIED BETWEEN TWO BARS OF DIFFERENT DIAMETERS, THE MINIMUM SPICE LENGTH SHALL BE THE GREATER OF THE SPICE LENGTH FOR THE SMALLER DIAMETER BAR AND THE EMBEDMENT LENGTH OF THE LARGER DIAMETER BAR.

WALLS

1. THESE NOTES APPLY SPECIFICALLY TO CONCRETE WALLS NOT CLASSIFIED AS SHEAR WALLS. SEE ALSO CONCRETE SHEAR WALL NOTES. UNLESS NOTED OTHERWISE, WALLS SHALL BE REINFORCED AS FOLLOWS:

WALL THICKNESS	FIRE EXPOSURE	VERTICAL REINFORCING	HORIZONTAL REINFORCING
6"	1 SIDE	10M @ 16" CENTERED	10M @ 13" CENTERED
	2 SIDES	10M @ 16" E.F. STAG.	10M @ 16" E.F. STAG.
8"	1 SIDE	10M @ 13" CENTERED	10M @ 10" OR 15M @ 20" CENTERED
	2 SIDES	10M @ 20" E.F. STAG.	10M @ 20" E.F. STAG.
10"	1 OR 2 SIDES	10M @ 20" E.F. STAG.	10M @ 16" E.F. STAG.
	1 OR 2 SIDES	10M @ 17" E.F.	10M @ 13" E.F.

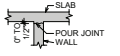
FOR OTHER THICKNESSES, REINFORCEMENT TO BE PROPORTIONAL TO ABOVE.

15M @ 20" MAY BE SUBSTITUTED FOR 10M @ 13" ONLY WITH THE APPROVAL OF RJC. FOR WALLS WITH A SINGLE LAYER OF STEEL, THE WALL REINFORCING SHALL BE PLACED IN THE CENTER OF THE WALL U.N.O.

REFER TO THE "CONCRETE REINFORCEMENT" NOTE AND THE ARCHITECTURAL DRAWINGS AND SPECIFICATIONS FOR FIRE EXPOSURE REQUIREMENTS. ALL WALLS DESIGNED FOR FIRE EXPOSURE ONE SIDE U.N.O. ON STRUCTURAL DRAWINGS.

2. PLACE VERTICAL REINFORCEMENT IN OUTER LAYERS OF THE CURTAINS AND HORIZONTAL REINFORCEMENT IN INNER LAYERS (BEHIND VERTICALS), UNLESS NOTED OTHERWISE.
3. ALL WALL REINFORCING SHALL BE CONTINUOUS, WITH HOOKS OR CORNER BARS USED AT ALL WALL JOINTS. EXTEND HOOKS TO FAR FACE OF WALL. CORNER BARS TO BE LOCATED ON OUTSIDE FACE OR CENTER OF WALL.
4. HORIZONTAL AND VERTICAL SPICES SHALL BE TENSION SPICES. U.N.O.
5. ENDS OF ALL WALLS AND ALL WALL INTERSECTIONS SHALL HAVE 2-15M VERTICAL MINIMUM UNLESS NOTED OTHERWISE ON DRAWINGS.
6. ADD 2-15M PARALLEL TO ALL EDGES AND EXTENDING 25" BEYOND CORNERS AT OPENINGS IN WALLS.
7. UNLESS NOTED OTHERWISE, PROVIDE DOWELS AT BOTTOM OF WALLS (E.G. AT FOOTINGS OR WHEREVER WALL BEGINS) AS SHOWN BELOW. DOWELS TO MATCH VERTICAL REINFORCEMENT.

8. UNLESS NOTED OTHERWISE, ALL RETAINING WALLS BELOW GRADE AND ALL EXTERIOR WALLS EXPOSED TO THE WEATHER ABOVE GRADE SHALL HAVE CONTROL JOINTS. SEE CONTROL JOINT DETAIL. CONSTRUCTION JOINT MAY REPLACE CONTROL JOINT WHERE DOWELS ARE NOT USED. THE LOCATION OF CONTROL JOINTS IN EXPOSED CONCRETE WALLS SHALL BE SUBMITTED TO THE ARCHITECT FOR REVIEW.
9. UNLESS NOTED OTHERWISE, PLACE TOP OF WALLS 0 TO 12" BELOW SOFFIT OF SUPPORTED CONCRETE STRUCTURE. IF TOP OF WALL PLACED ABOVE SOFFIT, CAREFULLY CHIP DOWN MINIMUM 3 DAYS AFTER ELEMENT POURED AND AS REQUIRED BY RJC.



EMBEDMENT / DEVELOPMENT LENGTHS AND SPICE LENGTHS

1. BASED ON CSA A23.3.
2. WHERE EMBEDMENT OR SPICES ARE DIMENSIONED ON THE DRAWINGS, SUCH DIMENSION SHALL APPLY.
3. WHERE THE DRAWINGS INDICATE A TENSION EMBEDMENT, IT IS A COMPRESSION EMBEDMENT LENGTH AND IT SHALL BE AS NOTED BELOW.
4. WHERE THE DRAWINGS INDICATE A TENSION EMBEDMENT, IT IS A TENSION EMBEDMENT LENGTH AND SHALL BE AS NOTED BELOW.
5. WHERE NO EMBEDMENT OR EMBEDMENT TYPE IS CALLED FOR ON THESE DRAWINGS, IT SHALL BE A TENSION EMBEDMENT, EXCEPT FOR COLUMNS WHICH SHALL BE A COMPRESSION EMBEDMENT.
6. WHERE NO SPICE OR SPICE TYPE IS CALLED FOR ON THESE DRAWINGS, IT SHALL BE A TENSION SPICE, EXCEPT FOR COLUMNS WHICH SHALL BE A COMPRESSION SPICE.
7. IN TABLES BELOW, EMBEDMENT LENGTHS ARE SHOWN WITHOUT BRACKETS, AND SPICE LENGTHS ARE SHOWN IN BRACKETS.
8. ALL TENSION SPICE LENGTHS ARE CLASS "B" (1.3 M).
9. WHERE MORE THAN ONE FACTOR APPLIES FOR INCREASING THE LENGTHS IN THESE TABLES, MULTIPLY ALL FACTORS TOGETHER.

COMPRESSION EMBEDMENT AND SPICE LENGTHS

10. COMPRESSION EMBEDMENT REFERS TO THE LENGTH REQUIRED TO PROVIDE THE "COMPRESSION DEVELOPMENT LENGTH" AS DEFINED IN CSA A23.3 CLAUSE 12.3.2.
11. SPICE LENGTH REFERS TO THE MINIMUM LAP LENGTH REQUIRED FOR A COMPRESSION SPICE AS DEFINED IN CSA A23.3 CLAUSE 12.6.1.

CONCRETE STRENGTH	FUNCTION	REBAR DESIGNATION (GRADE 400 LENGTHS)					
		10M	15M	20M	25M	30M	35M
20 MPa	EMBEDMENT	9"	13"	22"	26"	30"	37"
	(SPICE)	(12")	(18")	(23")	(29")	(35")	(41")
25 MPa	EMBEDMENT	8"	12"	16"	19"	23"	27"
	(SPICE)	(12")	(18")	(23")	(29")	(35")	(41")
30 MPa & GREATER	EMBEDMENT	8"	11"	14"	18"	21"	25"
	(SPICE)	(12")	(18")	(23")	(29")	(35")	(41")

NOTES:

1. TABLE SHOWS LENGTHS FOR GRADE 400 REINFORCEMENT. MULTIPLY VALUES BY 1.46 FOR GRADE 500 REINFORCEMENT.
2. WHERE A COMPRESSION SPICE IS SPECIFIED BETWEEN TWO BARS OF DIFFERENT DIAMETERS, THE MINIMUM SPICE LENGTH SHALL BE THE GREATER OF THE SPICE LENGTH FOR THE SMALLER DIAMETER BAR AND THE EMBEDMENT LENGTH OF THE LARGER DIAMETER BAR.

TENSION EMBEDMENT AND SPICE LENGTHS

12. TENSION EMBEDMENT REFERS TO THE LENGTH REQUIRED TO PROVIDE A "TENSION DEVELOPMENT LENGTH" AS DEFINED IN CAN/CSA-A23.3 CLAUSE 12.2.
13. SPICE LENGTH REFERS TO THE MINIMUM LAP LENGTH REQUIRED FOR A CLASS "B" TENSION SPICE (1.3M) AS PER CAN/CSA-A23.3 CLAUSE 12.15.

CASE 1 TENSION EMBEDMENT AND SPICE CONDITIONS

TENSION EMBEDMENT AND SPICE LENGTHS CONFORMING TO CSA A23.3 TABLE 12.1 (0.45 A_sA_uA_u / f_t) ARE TO BE AS PER THE FOLLOWING TABLE FOR:

- A. COLUMNS VERTS. REQUIRING TENSION SPICES.
- B. BEAM AND GIRDER TOP AND BOTTOM BARS.
- C. TWO WAY SLAB TOP AND BOTTOM BARS.
- D. ONE WAY SLAB BOTTOM BARS.
- E. WALL HORIZONTAL AND VERTICAL DISTRIBUTED REINFORCING.
- F. SEE ALSO NOTES ON TOP BARS.
- G. MEMBERS WHICH DO NOT SATISFY THE ABOVE CONDITIONS SHALL HAVE TENSION EMBEDMENTS AND SPICES AS PER CASE 2 TABLE BELOW.

CONCRETE STRENGTH	FUNCTION	REBAR DESIGNATION (GRADE 400 LENGTHS)					
		10M	15M	20M	25M	30M	35M
20 MPa	EMBEDMENT	13"	20"	26"	40"	48"	56"
	(SPICE)	(17")	(25")	(33")	(52")	(62")	(73")
25 MPa	EMBEDMENT	12"	18"	23"	36"	43"	50"
	(SPICE)	(16")	(23")	(30")	(47")	(56")	(65")
30 MPa	EMBEDMENT	12"	16"	21"	33"	39"	46"
	(SPICE)	(16")	(21")	(27")	(43")	(51")	(59")
35 MPa & GREATER	EMBEDMENT	12"	15"	20"	30"	36"	42"
	(SPICE)	(16")	(19")	(25")	(39")	(47")	(55")

NOTES:

1. TOP BAR VALUES ARE 1.3 TIMES THE ABOVE LENGTHS. TOP BAR APPLIES TO HORIZONTAL REINFORCEMENT CAST WITH 12" OR MORE OF CONCRETE BELOW THE BAR.
2. TOP BAR FACTOR DOES NOT APPLY TO HORIZONTAL WALL REINFORCEMENT IN WALLS THAT ARE NOT VIBRATED.
3. WHERE A TENSION SPICE IS SPECIFIED BETWEEN TWO BARS OF DIFFERENT DIAMETERS, THE MINIMUM SPICE LENGTH SHALL BE THE GREATER OF THE SPICE LENGTH FOR THE SMALLER DIAMETER BAR AND THE EMBEDMENT LENGTH OF THE LARGER DIAMETER BAR.

CONCRETE COVER

1. UNLESS NOTED OTHERWISE ON THE STRUCTURAL DRAWINGS, PROVIDE 2 HOUR FIRE RATING FOR ALL REINFORCED CONCRETE.
2. UNLESS OTHERWISE NOTED CONCRETE COVER TO REINFORCEMENT SHALL BE THE LARGEST OF A THROUGH H:
 - A. FOR FIRE RATINGS:

ELEMENT	FIRE RATINGS	
	0-2 HOURS	3 HOURS
	1 5/8" MIN. 2" MIN. TO VERTS.	2"
WALLS - NON-RETAINING AND EXPOSED TO FIRE ON 2 SIDES AND IDENTIFIED ON PLAN	1 5/8" MIN. 2" MIN. TO VERTS.	2"
WALLS - NON-RETAINING AND EXPOSED TO FIRE ON 1 SIDE	GREATER OF 3/4" AND 1.04L TO ZONE TIES / OUTER CURTAIN LAYER	2"
BEAMS	TO STIRRUPS	2"
NO STIRRUPS	2"	1 3/8"
SLABS	INSIDE FACE	GREATER OF 1" AND 1.5L
RETAINING / FOUNDATION WALLS (F-2 EXPOSURE)	GROUND OR EARTH SIDE	GREATER OF 1 5/8" AND 1.5L

PARKING AREAS

ELEMENT	FIRE RATINGS	
	0-2 HOURS	3 HOURS
	1 5/8" MIN. 2" MIN. TO VERTS.	2"
COLUMNS AND FORMED PILES	TO TIES	1 5/8" MIN. 2" MIN. TO VERTS.
WALLS - NON-RETAINING AND EXPOSED TO FIRE ON 2 SIDES AND IDENTIFIED ON PLAN	1 5/8" MIN. 2" MIN. TO VERTS.	2"
WALLS - NON-RETAINING AND EXPOSED TO FIRE ON 1 SIDE	GREATER OF 3/4" AND 1.04L TO ZONE TIES / OUTER CURTAIN LAYER	2"
BEAMS, GIRDERS & TRANSFER SLABS WITH MEMBRANE	TO STIRRUPS	1 5/8"
NO STIRRUPS	2"	1 5/8"
SLABS AND SLAB BANDS, STIRRUPS IN SLAB BANDS WITH MEMBRANE	TOP COVER	GREATER OF 1 5/8" OR 1.5L
BOT. COVER	GREATER OF 1 1/4" AND 1.5L	GREATER OF 1 3/8" AND 1.5L
STRUCTURAL SLAB ON GRADE TOP AND BOTTOM COVER	2 1/4"	N/A
RETAINING / FOUNDATION WALLS COVER BOTH FACES (C-1 EXPOSURE)	GREATER OF 1 5/8" AND 1.5L	N/A

- B. UNLESS NOTED OTHERWISE IN NOTES C THROUGH H, MINIMUM CONCRETE COVER BASED ON REINFORCEMENT SIZE (db) AND COARSE AGGREGATE SIZE (ag)
 - A. CONCRETE CAST AGAINST EARTH OR GROUND
 - B. CONCRETE WITH NO MEMBRANE (NON-PARKING) AND EXPOSED TO CHLORIDES - EXPOSURE CLASS CXL C1, C1 AND C3.
 - C. FORMED FINISHED CONCRETE EXPOSED TO WEATHER - EXPOSURE CLASS F1, F2, S1, S2, OR EARTH.
 - D. CONCRETE IN PARKING AREAS, BOTTOM BARS.

SEE ARCHITECTURAL, DRAWINGS AND STRUCTURAL DRAWINGS FOR AREAS WHICH MAY REQUIRE 3 OR 4 HOUR RATINGS.
SEE STRUCTURAL DRAWINGS FOR AREAS CLASSIFIED AS (D) OR (E) ABOVE FOR WEATHER EXPOSURE.

DESIGNATION OF REINFORCING BARS

1. BARS SHOWN THUS ——— IN BOTTOM OF BEAMS OR SLABS OR IN FAR FACE OF WALL.
2. BARS SHOWN THUS ——— IN TOP OF BEAMS OR SLABS OR IN NEAR FACE OF WALL.

STRAIGHT BARS:

6-10M13'S MEANS 6-10M BARS 13'-0" LONG.
10M12'S @ 12" STAG. 2'-0" MEANS 2'-0" OFFSET FOR EACH 20M 12" 1" BAR SPACED AT 12" O.C.
IF STAGGER NOT SPECIFIED SEE GENERAL NOTES AND DETAIL. DETAILS FOR DIMENSION.

6-10M13'S @ 12" STAG. 2'-0" MEANS 2'-0" OFFSET FOR EACH 20M 12" 1" BAR SPACED AT 12" O.C.
IF STAGGER NOT SPECIFIED SEE GENERAL NOTES AND DETAIL. DETAILS FOR DIMENSION.

6-10M13'S @ 12" STAG. 2'-0" MEANS 2'-0" OFFSET FOR EACH 20M 12" 1" BAR SPACED AT 12" O.C.
IF STAGGER NOT SPECIFIED SEE GENERAL NOTES AND DETAIL. DETAILS FOR DIMENSION.

6-10M13'S @ 12" STAG. 2'-0" MEANS 2'-0" OFFSET FOR EACH 20M 12" 1" BAR SPACED AT 12" O.C.
IF STAGGER NOT SPECIFIED SEE GENERAL NOTES AND DETAIL. DETAILS FOR DIMENSION.

6-10M13'S @ 12" STAG. 2'-0" MEANS 2'-0" OFFSET FOR EACH 20M 12" 1" BAR SPACED AT 12" O.C.
IF STAGGER NOT SPECIFIED SEE GENERAL NOTES AND DETAIL. DETAILS FOR DIMENSION.

6-10M13'S @ 12" STAG. 2'-0" MEANS 2'-0" OFFSET FOR EACH 20M 12" 1" BAR SPACED AT 12" O.C.
IF STAGGER NOT SPECIFIED SEE GENERAL NOTES AND DETAIL. DETAILS FOR DIMENSION.

6-10M13'S @ 12" STAG. 2'-0" MEANS 2'-0" OFFSET FOR EACH 20M 12" 1" BAR SPACED AT 12" O.C.
IF STAGGER NOT SPECIFIED SEE GENERAL NOTES AND DETAIL. DETAILS FOR DIMENSION.

6-10M13'S @ 12" STAG. 2'-0" MEANS 2'-0" OFFSET FOR EACH 20M 12" 1" BAR SPACED AT 12" O.C.
IF STAGGER NOT SPECIFIED SEE GENERAL NOTES AND DETAIL. DETAILS FOR DIMENSION.

6-10M13'S @ 12" STAG. 2'-0" MEANS 2'-0" OFFSET FOR EACH 20M 12" 1" BAR SPACED AT 12" O.C.
IF STAGGER NOT SPECIFIED SEE GENERAL NOTES AND DETAIL. DETAILS FOR DIMENSION.

6-10M13'S @ 12" STAG. 2'-0" MEANS 2'-0" OFFSET FOR EACH 20M 12" 1" BAR SPACED AT 12" O.C.
IF STAGGER NOT SPECIFIED SEE GENERAL NOTES AND DETAIL. DETAILS FOR DIMENSION.

6-10M13'S @ 12" STAG. 2'-0" MEANS 2'-0" OFFSET FOR EACH 20M 12" 1" BAR SPACED AT 12" O.C.
IF STAGGER NOT SPECIFIED SEE GENERAL NOTES AND DETAIL. DETAILS FOR DIMENSION.

6-10M13'S @ 12" STAG. 2'-0" MEANS 2'-0" OFFSET FOR EACH 20M 12" 1" BAR SPACED AT 12" O.C.
IF STAGGER NOT SPECIFIED SEE GENERAL NOTES AND DETAIL. DETAILS FOR DIMENSION.

6-10M13'S @ 12" STAG. 2'-0" MEANS 2'-0" OFFSET FOR EACH 20M 12" 1" BAR SPACED AT 12" O.C.
IF STAGGER NOT SPECIFIED SEE GENERAL NOTES AND DETAIL. DETAILS FOR DIMENSION.

6-10M13'S @ 12" STAG. 2'-0" MEANS 2'-0" OFFSET FOR EACH 20M 12" 1" BAR SPACED AT 12" O.C.
IF STAGGER NOT SPECIFIED SEE GENERAL NOTES AND DETAIL. DETAILS FOR DIMENSION.

6-10M13'S @ 12" STAG. 2'-0" MEANS 2'-0" OFFSET FOR EACH 20M 12" 1" BAR SPACED AT 12" O.C.
IF STAGGER NOT SPECIFIED SEE GENERAL NOTES AND DETAIL. DETAILS FOR DIMENSION.

6-10M13'S @ 12" STAG. 2'-0" MEANS 2'-0" OFFSET FOR EACH 20M 12" 1" BAR SPACED AT 12" O.C.
IF STAGGER NOT SPECIFIED SEE GENERAL NOTES AND DETAIL. DETAILS FOR DIMENSION.

6-10M13'S @ 12" STAG. 2'-0" MEANS 2'-0" OFFSET FOR EACH 20M 12" 1" BAR SPACED AT 12" O.C.
IF STAGGER NOT SPECIFIED SEE GENERAL NOTES AND DETAIL. DETAILS FOR DIMENSION.

6-10M13'S @ 12" STAG. 2'-0" MEANS 2'-0" OFFSET FOR EACH 20M 12" 1" BAR SPACED AT 12" O.C.
IF STAGGER NOT SPECIFIED SEE GENERAL NOTES AND DETAIL. DETAILS FOR DIMENSION.

1	ISSUED FOR 50% REVIEW	2024.12.20	KML
No.	Revision	Date	By

Drawing Notes

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Seal

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Project Name

PROPOSED COLLEGE DRIVE APARTMENTS

1202 COLLEGE DRIVE SASKATOON, SK

Sheet Title

GENERAL NOTES

Drawn By **KML** Scale **As Indicated**

Designed By **PWM** Date **2024.12.20**

RJC Project Number **SAS.138936.0001**

Sheet Number **1** Revision

S 1.4

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HEADED STUD ASSEMBLY CONFIGURATIONS

ALSO SEE "HEADED STUD ASSEMBLIES" NOTES.

TYPE 2A - ROUND COLUMN

TYPE 2A - SQUARE/RECTANGULAR COL.

TYPE 2B

TYPE 2E

TYPE 3A - ROUND COLUMN

TYPE 3A - SQUARE/RECTANGULAR COL.

TYPE 3C

TYPE 3E

TYPE 4A - ROUND COLUMN

TYPE 4A - SQUARE/RECTANGULAR COL.

TYPE 4B

TYPE 4C

TYPICAL CORNER MAT LAYOUT FLAT SLAB

NOTES:

- MAX ONE CARRY BAR AND ONE CONCENTRATED BAR IN T.U.L. DIRECTION MAY BE DROPPED TO T.L.L.

TYPICAL SLAB EDGE HOOK BARS

CASE 1

CASE 2

HEADED STUD ASSEMBLIES

1. TYPICAL HEADED STUD ASSEMBLY DETAIL

2. MATERIALS AND MANUFACTURING SHALL BE IN ACCORDANCE WITH THE LATEST UPDATE OF ASTM A1044.

3. HEADED STUDS SHALL CONFORM TO ASTM A29 GRADES 1010 THROUGH 1020 AND THE FOLLOWING TENSILE PROPERTIES:

D	CROSS-SECTION AREA OF STUD	Dh	th	br	tr
3/8"	0.110 SQ. INCHES	1.19"	0.21"	1"	3/16"
1/2"	0.196 SQ. INCHES	1.58"	0.28"	1 1/4"	1/4"
5/8"	0.307 SQ. INCHES	1.98"	0.35"	1 3/4"	5/16"
3/4"	0.442 SQ. INCHES	2.37"	0.42"	2"	3/8"

4. BASE RAILS SHALL CONFORM TO ASTM A36 AND THE FOLLOWING TENSILE PROPERTIES:

	TENSILE STRENGTH =	450 MPa (65000 PSI) MINIMUM
	YIELD STRENGTH = <td>345 MPa (50000 PSI) MINIMUM</td>	345 MPa (50000 PSI) MINIMUM
	ELONGATION IN 2" = <td>20% MINIMUM</td>	20% MINIMUM
	REDUCTION OF AREA = <td>50% MINIMUM</td>	50% MINIMUM

5. STUD WELDING SHALL CONFORM TO CSA W59, INCLUDING THE PROVISIONS FOR PRODUCTION CONTROL, FABRICATION, AND VERIFICATION REQUIREMENTS.

6. TESTING AND INSPECTION OF HEADED STUD ASSEMBLIES SHALL BE IN ACCORDANCE WITH THE STRUCTURAL SPECIFICATIONS.

7. COVER OF THE HEADED STUD ASSEMBLY TO THE TOP AND BOTTOM SLAB SURFACES ARE AS PER THE CONCRETE REINFORCEMENT NOTES.

8. ALL HEADED STUD ASSEMBLIES TO BE INSTALLED WITH THE RAIL LOCATED BELOW THE LOWEST LAYER OF REINFORCEMENT UNLESS NOTED OTHERWISE ON DRAWINGS OR APPROVED IN WRITING BY RJC.

9. ALLOW MINIMUM 2" CLEAR COVER FROM THE HEADED STUD ASSEMBLY TO SLAB EDGES UNLESS NOTED OTHERWISE ON DRAWINGS.

10. THE ALL HEADED STUD ASSEMBLIES TO REINFORCING BARS AND USE ADEQUATE CHAIRS TO PREVENT MOVEMENT OF THE HEADED STUD ASSEMBLIES DURING CASTING.

11. NO IN-SLAB DUCTS PERMITTED IN THE REGION CONTAINING HEADED STUD ASSEMBLIES UNLESS APPROVED IN WRITING BY RJC.

12. NO SLEEVES OR CONDUIT PERMITTED WITHIN THE REGION CONTAINING HEADED STUD ASSEMBLIES UNLESS APPROVED IN WRITING BY RJC.

SLAB TEMPERATURE REINFORCING

1. MINIMUM BOTTOM (UNLESS NOTED OTHERWISE):

SLAB THICKNESS	TEMPERATURE REINFORCING	SLAB THICKNESS	TEMPERATURE REINFORCING
5"	10M @ 16"	8"	15M @ 12"
5 1/2"	10M @ 14"	8 1/2"	15M @ 10"
6"	10M @ 13"	9"	15M @ 17"
6 1/2"	10M @ 11"	10"	15M @ 14"
7"	10M @ 20"	11"	15M @ 13"
7 1/2"	10M @ 20"	12"	15M @ 13"

2. FOR OTHER THICKNESSES REINFORCEMENT TO BE PROPORTIONAL TO ABOVE.

3. BASED ON CAN/CSA-A23.3: 0.002 X AREA.

4. 15M BOTTOM BARS MAY BE REPLACED WITH 10M BARS AT CLOSER SPACING SATISFYING 0.002 X AREA. (EG. REPLACE 15M @ 20" WITH 10M @ 10"). DO NOT INCREASE BAR SPACING UNLESS APPROVED BY RJC.

INTEGRITY REINFORCEMENT

1. PROVIDE INTEGRITY REINFORCEMENT IN ALL FLAT PLATES, FLAT SLABS WITH DROPS, AND SLABS WITH SLAB BANDS AS PER THE NOTE (SCHEDULE SHOWN ON THE PLAN SHEETS UNLESS NOTED OTHERWISE ON PLAN.

2. SEE S 3.1 FOR TYPICAL DETAILS OF INTEGRITY REINFORCEMENT PLACEMENT.

3. ALL INTEGRITY REINFORCEMENT TO BE CONTINUOUS BARS WITHOUT SPLICES AND ALL BARS MUST PASS THROUGH THE COLUMN CORE. ADJUST SLEEVE LOCATIONS TO SUIT INTEGRITY REINFORCEMENT LAYOUT.

4. CENTER ALL INTEGRITY BARS ABOUT COLUMN CENTERLINE. WHERE SLAB EDGE OR EDGE OF OPENING IS LOCATED LESS THAN ONE-HALF OF THE INTEGRITY BAR LENGTH FROM COLUMN CENTERLINE OR LESS THAN 2" WHERE APPLICABLE, EXTEND THE INTEGRITY REINFORCEMENT TO SLAB EDGE LESS COVER AND PROVIDE STANDARD HOOK OR BAR TERMINATOR (5 X 4 HEAD).

TYPICAL MAT LAYOUT WITHOUT DROP PANEL

NOTES:

- CONCENTRATE EXTRA TOP BARS IN MIDDLE 1/3 OF MAT
- MAX TWO CARRY BARS + ONE CONCENTRATED BAR IN T.U.L. DIRECTION MAY BE DROPPED TO T.L.L.

TYPICAL EDGE MAT LAYOUT FLAT SLAB

NOTES:

- WHERE T.U.L. PARALLEL TO SLAB EDGE, MAX ONE CARRY BAR AND ONE CONCENTRATED BAR IN T.U.L. DIRECTION MAY BE DROPPED TO T.L.L.
- WHERE T.U.L. PERPENDICULAR TO SLAB EDGE, MAX ONE T.U.L. BAR EACH SIDE OF COLUMN MAY BE DROPPED TO T.L.L. DO NOT DROP BASIC BARS PERPENDICULAR TO SLAB EDGE U.O.

COLUMN TIE ARRANGEMENTS

TYPE 1

TYPE 2

TYPE 3

TYPE 4

NOTES:

- MAXIMUM CLEAR DISTANCE BETWEEN VERTICAL BARS ENCLOSED BY THE CORNER OF A TIE, AND WITHOUT AN INTERMEDIATE BAR, IS 20".
- MAXIMUM ONE BAR MAY BE PLACED BETWEEN TIED BARS. MAXIMUM CLEAR DISTANCE BETWEEN TIED VERTICAL BARS FOR THIS CASE IS 12".
- ALL TIES TO HAVE 135° HOOKS AND BE BUCKING PREVENTION TIES DETAILED AS HOOPS WITH SEISMIC HOOPS UNLESS NOTED OTHERWISE.
- CLOSED TIES MAY ALWAYS BE SUBSTITUTED FOR CROSS-TIES.
- ROUND OR OCTAGONAL COLUMNS TO HAVE 6 VERTICAL BARS MINIMUM. OFFSET LOCATION OF THE HOOKS OVER HEIGHT OF COLUMN.

SLABS

1. UNLESS NOTED OTHERWISE, THE MINIMUM BOTTOM REINFORCING IN BOTH DIRECTIONS IN SLABS SHALL BE AS SHOWN ON SLAB TEMPERATURE REINFORCING NOTES.

2. UNLESS NOTED OTHERWISE, EDGES OF ALL SLABS SHALL HAVE 1-15M TOP + 1-15M BOTTOM CONTINUOUS LAPPED 25". AT RE-ENTRANT CORNER EXTEND 2'-1" BEYOND THE CORNER. STAGGER TOP AND BOTTOM BAR LAPS MINIMUM 4'-0".

3. CAMBERS: FOR SLABS, BEAMS, GIRDERS

A. CIRCLED NUMBERS, E.G. (6) INDICATES POSITION AND MAGNITUDE OF POINTS WHERE SLABS SHALL BE CAMBERED 5/8".

B. FOR SPANS OVER 23'-0" WHEN CAMBERS ARE NOT INDICATED ON DRAWINGS, SPANS SHALL BE CAMBERED 0.002 OF SPAN.

C. UNLESS NOTED OTHERWISE, POST-TENSIONED SLABS AND BEAMS NEED NOT BE CAMBERED.

4. LOCATIONS AND DETAILS OF CONSTRUCTION JOINTS TO BE SUBMITTED TO RJC FOR REVIEW AND APPROVAL PRIOR TO CONSTRUCTION.

5. UNLESS NOTED OTHERWISE, SLAB TEMPERATURE REINFORCING SHALL BE TENSION SPICED. SEE "EMBEDMENT / DEVELOPMENT LENGTHS AND SPICE LENGTHS" NOTE.

6. WHERE NOTED ON PLAN SLAB RECESSES TO HAVE SLAB THICKNESS LOCALLY REDUCED.

7. OPENINGS IN SLABS:

A. UNLESS NOTED OTHERWISE, SLAB REINFORCING SHALL NOT BE CUT AT OPENINGS INCLUDING SLEEVES, PLUMBING BLOCKS OUT AND EMBEDDED ELECTRICAL BOXES.

B. AT OPENINGS UP TO 12" X 12", SPREAD REINFORCING AROUND THE OPENING.

C. AT OPENINGS UP TO 2'-6" X 2'-6", DISPLACE SLAB REINFORCEMENT AROUND OPENING AND PROVIDE EXTRA REINFORCEMENT AS SHOWN UNLESS NOTED OTHERWISE ON PLANS AND DETAILS.

NOT FOR CONSTRUCTION

NOTES:

- AT OPENINGS GREATER THAN 2'-6", SEE PLANS AND DETAILS FOR REINFORCEMENT.
- NO OPENINGS PERMITTED WITHIN TWICE THE SLAB DEPTH FROM THE FACE OF COLUMN OR EDGE OF CAPITAL, EXCEPT AS NOTED ON THE STRUCTURAL DRAWINGS.
- ALSO SEE "CONDUITS, PIPES, AND SLEEVES EMBEDDED IN CONCRETE" NOTE AND THE SPECIFICATIONS.

NOT FOR CONSTRUCTION

Project Name
**PROPOSED COLLEGE
DRIVE APARTMENTS**

1202 COLLEGE DRIVE
SASKATOON, SK

Sheet Title
GENERAL NOTES

Drawn By
Designed By
RJC Project Number
Sheet Number

Scale
As Indicated
Date
2024.12.20
SAS.138936.0001
Revision

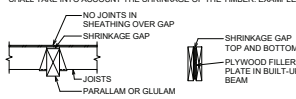
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WOOD MOVEMENT DUE TO SHRINKAGE		WOOD FRAMING - MATERIALS		WOOD FRAMING - GENERAL		NON-LOAD BEARING MASONRY DELEGATED DESIGN																				
<p>1. THE WOOD FRAME STRUCTURE WILL UNDERGO MOVEMENT DUE TO SHRINKAGE. SHRINKAGE OCCURS AS THE MOISTURE CONTENT IN WOOD DROPS AS IT IS DRIED IN SERVICE. THE MOISTURE CONTENT SHOULD REACH AROUND 6% AND IS CONSIDERED THE EQUILIBRIUM MOISTURE (EMC) CONTENT FOR THE PROJECT.</p> <p>2. KILN DRIED WOOD IS INITIALLY DRIED TO AN AVERAGE MOISTURE CONTENT OF 15% WITH NO PIECE EXCEEDING 19%.</p> <p>3. WOOD MAY TAKE ON MOISTURE DURING CONSTRUCTION DUE TO WEATHER AND EXCEED THE INITIAL FABRICATION MOISTURE CONTENT.</p> <p>4. THE CONTRACTOR IS TO TAKE ALL REASONABLE MEASURES TO PROTECT WOOD FROM MOISTURE WHILE IT IS STORED ON SITE. THIS INCLUDES STORING LEVEL ON THE SITE, RAISED OFF THE GROUND, STACKED USING SEPARATING SPACERS, AND STORED IN A DRY AREA OR PROTECTED WITH A MOISTURE RESISTANT COVER. THE CONTRACTORS SHOULD ALSO TAKE REASONABLE MEASURES TO CLOSE IN THE STRUCTURE AS QUICKLY AS POSSIBLE, REDUCING THE EXPOSURE OF WOOD TO MOISTURE DURING CONSTRUCTION. WOOD ELEMENTS ALONG THE PERIMETER OF THE BUILDING SHOULD BE COVERED TO PROTECT THEM FROM RAIN. IN THE EVENT THAT THERE IS POOLED WATER FROM RAINFALL ON STRUCTURE, MEASURES SHOULD BE TAKEN TO REMOVE THE WATER PROPERLY TO AVOID SATURATION OF THE WOOD.</p> <p>5. FOR THE PROJECT, GLULAM BEAMS AND WALL PLATES ARE SPECIFIED TO HAVE A MAXIMUM MOISTURE CONTENT OF 15% AT TIME OF FABRICATION. CLT PANELS TO MEET PNG 30 REQUIREMENTS AND WILL BE LESS THAN 15%, BUT HAVE BEEN ASSUMED 15% FOR PURPOSES OF CALCULATING SHRINKAGE.</p> <p>6. THE CONTRACTOR IS TO TAKE ALL REASONABLE MEANS TO KEEP THE BUILDING MOISTURE CONTENT OF THE WOOD STRUCTURE TO 15% MAXIMUM DURING CONSTRUCTION. THIS INCLUDES CONSTRUCTING IN THE DRY SEASON, TARPING WHEN POSSIBLE, REMOVING STANDING AND/OR TRAPPED WATER, OR OTHER REASONABLE MEASURES.</p> <p>7. PRIOR TO ENCLOSING THE STRUCTURE WITH THE BUILDING ENVELOPE, THE CONTRACTOR IS TO DRY THE WOOD TO A MAXIMUM MOISTURE CONTENT OF 15%. THIS INCLUDES ALL WOOD IN THE STRUCTURE (NOT JUST THE EXTERIOR WALL).</p> <p>8. THE CONTRACTOR IS TO ENGAGE A BUILDING ENVELOPE SPECIALIST TO MONITOR REPRESENTATIVE VALUES OF WOOD WITHIN THE BUILDING TO ENSURE MAXIMUM MOISTURE OF 15% IS NOT EXCEEDED, AFTER WHICH TIME BUILDING FINISHES CAN BE PLACED.</p> <p>9. THE BUILDING WILL HAVE TWO INITIAL STAGES OF CHANGE IN MOISTURE CONTENT IN THE WOOD. THE FIRST BEING THE CHANGE IN MOISTURE CONTENT UNTIL WHICH TIME THE STRUCTURE IS DRIED TO 15%. SHOULD IT TAKE ON MOISTURE DURING CONSTRUCTION, IT IS ASSUMED INITIALLY THE WOOD MAY BE AT 24% MOISTURE CONTENT AS AN UPPER LIMIT. THE SECOND BEING THE CHANGE FROM 15% MOISTURE CONTENT TO THE EQUILIBRIUM MOISTURE CONTENT OF 6% WHICH IS THE ANTICIPATED FINAL MOISTURE CONTENT EXPECTED.</p> <p>10. THE NATURAL MECHANICAL CONDITIONING STRATEGY FOR THE PROJECT WILL RESULT IN VARIABLE LEVELS OF RELATIVE HUMIDITY (RH) THROUGH THE SERVICE LIFE OF THE BUILDING, WITH RANGES ANTICIPATED IN WINTER BETWEEN 12% AND 18% AND IN SUMMER BETWEEN 50% AND 80%. THE WOOD STRUCTURE WILL FUNCTION AS A PASSIVE CONDITIONING AGENT OVER TIME, TAKING IN MOISTURE AS THE RH INCREASES AND RELEASING IT AS THE RH DECREASES. THE ANTICIPATED RANGES OF RH IN COMBINATION WITH TEMPERATURE EQUATE TO PROBABLE CHANGES IN EQUILIBRIUM MOISTURE CONTENT (EMC) OF 2% TO 20% WHICH MAY RESULT IN CHECKING AND SURFACE CRACKS. REGULAR MONITORING OF THE BUILDING RH, ELEMENT EMC, AND ANY VISUAL DISTRESSES IS IMPORTANT AND RECOMMENDED.</p> <p>11. IT IS THE CONTRACTOR'S RESPONSIBILITY TO ENSURE THAT ALL TRADES ARE AWARE OF THE POTENTIAL PER FLOOR SHRINKAGE AND CUMULATIVE SHRINKAGE THAT MAY OCCUR IN THE BUILDING AT VARIOUS STAGES OF CONSTRUCTION AND HOW MUCH THEY NEED TO ACCOMMODATE FOR IN THEIR DETAILS AND CONNECTIONS. THE CONTRACTORS SHOULD ALSO BE AWARE OF THE GENERAL MOISTURE CONTENT OF PLATES, CLT PANELS, AND BEAMS SHOULD THEY BE SUBJECT TO SIGNIFICANT MOISTURE DURING CONSTRUCTION.</p> <p>12. FOR SYSTEMS OR COMPONENTS THAT ARE INSTALLED BEFORE THE BUILDING HAS BEEN DRIED TO 15%, ALLOW FOR THE FULL SHRINKAGE FROM 24% TO 10%. FOR SYSTEMS OR COMPONENTS THAT ARE ADDED AFTER THE BUILDING HAS BEEN DRIED TO 15%, ALLOW FOR THE SHRINKAGE BETWEEN 15% AND 6%. IF THE MOISTURE CONTENT SHOULD BE FOUND TO BE GREATER THAN 24% DURING CONSTRUCTION, CONTACT RJC FOR REVERSED SHRINKAGE VALUES FOR COMPONENTS THAT MAY BE AFFECTED.</p> <p>13. ALL VERTICAL WOOD FRAMING IS CONSIDERED BALLOON FRAMING RESULTING IN NEGLIGIBLE CUMULATIVE SHRINKAGE THROUGH HEIGHT OF BUILDING. SHRINKAGE WILL OCCUR WITHIN FLOOR ASSEMBLY ITSELF.</p> <p>14. THE SHRINKAGE OR SWELLING OF A WOOD MEMBER BETWEEN THE INITIAL AND FINAL MOISTURE CONTENT MAY BE ESTIMATED BY USING THE EQUATION OUTLINED IN CSA 086 AS 4.6.</p>		<p>1. STUDS AND BUILT-UP POSTS TO BE S-P-F NO 1/NO 2 GRADE OR BETTER. EQUIVALENT CAN BE FINGER-JOINTED (MAXIMUM 3 JOINTS PER STUD) EXCEPT IN SHEAR WALLS. REFER TO WOOD SHEAR WALL NOTES FOR ADDITIONAL REQUIREMENTS. FINGER-JOINTED STUDS IN FIRE SEPARATIONS SHALL HAVE HEAT RESISTANT ADHESIVE (HSA).</p> <p>2. BUILT-UP BEAMS AND HEADERS TO BE S-P-F NO 1/NO 2 GRADE OR BETTER.</p> <p>3. WALL PLATES TO BE S-P-F NO 1/NO 2 GRADE OR BETTER. REFER TO WOOD SHEAR WALL NOTES FOR ADDITIONAL REQUIREMENTS. WALL PLATES SHALL BE KILNDRIED AND MAY BE FINGER-JOINTED EXCEPT IN SHEAR WALLS.</p> <p>4. POSTS AND BEAMS TO BE S-P-F NO 2 GRADE OR BETTER.</p> <p>5. CLT - CROSS-LAMINATED TIMBER PANELS PER CLT NOTES.</p> <p>6. ALL DIMENSION LUMBER TO BE SURFACED FOUR SIDES ("S4S").</p> <p>7. PLYWOOD TO BE DOUGLAS FIR PLYWOOD (DFF). REGULAR GRADES OF UNSANDED, CANADIAN SOFTWOOD PLYWOOD (CSP). REGULAR GRADES OF UNSANDED.</p> <p>8. O.S.B. TO CONFORM TO CSA 0325.</p> <p>9. TIMBER CONNECTION HARDWARE TO BE SIMPSON STRONG-TIE, OR EQUIVALENT APPROVED BY RJC. COMPLETE WITH NAILS SUPPLIED BY MANUFACTURER. DO NOT USE U.NAILS.</p> <p>10. NAILS - SEE "WOOD FRAMING - NAILING".</p> <p>11. MISCELLANEOUS STEEL TO BE CSA G40.21 OR APPROVED EQUIVALENT.</p> <p>12. ANCHOR RODS SHALL BE ASTM F1554 GRADE 36 OR APPROVED EQUIVALENT. ANCHOR RODS SHALL BE BEFORE PROTECTED WITH A MOISTURE RESISTANT COVER. THE CONTRACTOR SHOULD ALSO TAKE REASONABLE MEASURES TO CLOSE IN THE STRUCTURE AS QUICKLY AS POSSIBLE TO REDUCE THE EXPOSURE OF WOOD TO MOISTURE DURING CONSTRUCTION. BEAMS IN BALCONIES SHOULD ALSO BE COVERED AS LONG AS POSSIBLE.</p> <p>13. BOLTS SHALL BE ASTM A307 OR APPROVED EQUIVALENT. USED WITH STANDARD CUT STEEL WASHERS UNLESS NOTED OTHERWISE ON DRAWINGS.</p> <p>14. MOISTURE CONTENT OF ALL TIMBER ELEMENTS SHALL NOT EXCEED 19% AT THE TIME OF CONSTRUCTION OR FABRICATION.</p> <p>15. ALL FASTENERS AND CONNECTION HARDWARE THROUGH PRESERVATIVE TREATED MATERIALS OR OUTSIDE OF THE MOISTURE BARRIER TO BE NOT DIPPED GALVANIZED OR STAINLESS STEEL AS SPECIFIED.</p>		<p>1. ALL LOADS AND DESIGN SHALL CONFORM TO PART 4 OF THE NATIONAL BUILDING CODE OF CANADA. SEE "DESIGN LOADS" NOTE. ALL DETAILS, MATERIALS, NAILING, AND CONSTRUCTION PROCEDURES SHALL CONFORM TO PART 5 AS A MINIMUM.</p> <p>2. ALL LOADS, DESIGN, DETAILS, MATERIALS, NAILING, AND CONSTRUCTION PROCEDURES SHALL CONFORM TO PART 9 OF THE NATIONAL BUILDING CODE OF CANADA. SEE "DESIGN LOADS" NOTE.</p> <p>3. ALL DESIGN DETAILS, MATERIALS AND CONSTRUCTION PROCEDURES SHALL ALSO CONFORM TO CURRENT EDITIONS OF THE FOLLOWING AS A MINIMUM:</p> <ul style="list-style-type: none">- CSA 086 - ENGINEERING DESIGN IN WOOD- CSA 0121 - DOUGLAS FIR PLYWOOD- CSA 0151 - CANADIAN SOFTWOOD PLYWOOD- CAN/CSA-0122 - STRUCTURAL GLUED-LAMINATED TIMBER- CSA 0177 - QUALIFICATION CODE FOR MANUFACTURERS OF STRUCTURAL GLUED-LAMINATED TIMBER- CSA 0437 SERIES - STANDARDS FOR OSB AND WATERBOARD- CSA 0111 - WIRE NAILS, SPIKES AND STAPLES- ASTM D5456 - STANDARD SPECIFICATION FOR EVALUATION OF STRUCTURAL COMPOSITE LUMBER PRODUCTS- CANADIAN WOOD-FRAME HOUSE CONSTRUCTION CMHC "WOOD DESIGN MANUAL" - CANADIAN WOOD COUNCIL- "WOOD BUILDING TECHNOLOGY" - CANADIAN WOOD COUNCIL <p>4. ANY CHANGES TO THE FRAMING SHOWN ON THESE DRAWINGS SHALL HAVE PRIOR WRITTEN APPROVAL OF RJC. FRAMING CHANGES WHICH HAVE NOT BEEN SO APPROVED WILL BE REJECTED.</p> <p>5. CONFIRM ALL DIMENSIONS AND OUTLINES WITH THE ARCHITECTURAL DRAWINGS. SEE ARCHITECTURAL DRAWINGS FOR ADDITIONAL DIMENSIONS, ELEVATIONS AND DETAILS.</p> <p>6. ANY TIMBER NOT GRADE MARKED WILL BE REJECTED.</p> <p>7. FINISHES SHALL BE DETAILED TO ACCOMMODATE SHRINKAGE OF THE TIMBER OVER TIME.</p> <p>8. DO NOT COVER WOOD FRAMING WITH FINISHES UNTIL RJC'S FRAMING REVIEW IS COMPLETE. PROVIDE 24 HOURS ADVANCE NOTIFICATION WHEN FRAMING REVIEWS ARE REQUIRED.</p> <p>9. NOTCHING AND DRILLING OF STRUCTURAL ELEMENTS SHALL FOLLOW THE GUIDELINES SET FORTH IN THE BUILDING CODE PART 9, UNLESS OTHERWISE APPROVED IN WRITING BY RJC.</p> <p>10. ALL TIMBER ELEMENTS ARE DESIGNED FOR DRY-SERVICE CONDITIONS UNLESS NOTED OTHERWISE. SEE ARCHITECTURAL DRAWINGS FOR WATERPROOFING AND VENTILATION DETAILS.</p> <p>11. ALL WOOD FRAME CONSTRUCTION SHALL SATISFY THE FOLLOWING CONSTRUCTION TOLERANCES AS A MINIMUM. REFER TO ARCHITECTURAL AND WARRANTY REQUIREMENTS FOR ADDITIONAL TOLERANCE SPECIFICATIONS.</p> <ul style="list-style-type: none">A. FLOORS - NOT MORE THAN 1/4" IN 10' 0" OUT OF LEVEL.B. WALLS - NOT MORE THAN 1/4" IN 8' 0" OUT OF PLUMB, NOT MORE THAN 1/4" IN 10' 0" FOR ANY BOWING.C. OVERALLS - BUILDING WALLS AND FLOORS SHALL NOT BE MORE THAN 3/8" DIFFERENCE IN MEASUREMENT FROM DIMENSIONS SHOWN ON CONTRACT DOCUMENTS.		<p>1. UNLESS NOTED OTHERWISE ALL NON-LOAD BEARING MASONRY TO BE DESIGNED BY SPECIALTY ENGINEER IN ACCORDANCE WITH THE SPECIFICATIONS.</p> <p>2. [SUBMIT SCHEDULES S-B AND S-C FOR DESIGN OF ALL MASONRY TO THE ARCHITECT]</p> <p>3. THE FOLLOWING NOTES FOR NON-LOAD BEARING MASONRY ARE PROVIDED FOR INITIAL PRICE ESTIMATING PURPOSES AND AS AN OUTLINE. THESE NOTES SHALL NOT BE CONSIDERED PART OF THE CONSTRUCTION DOCUMENTS.</p> <p>4. UNDER AWARD OF THE MASONRY CONTRACT THE SUCCESSFUL CONTRACTOR SHALL ASSUME FULL RESPONSIBILITY FOR THE ENGINEERING DESIGN OF THE NON-LOAD BEARING MASONRY AS DEPICTED ON THE ARCHITECTURAL DRAWINGS AND SHALL ALSO COORDINATE THE DESIGN OF THE MASONRY WITH MECHANICAL AND ELECTRICAL TRADES FOR WALL OPENINGS.</p> <p>5. NON-LOAD BEARING PARTITIONS SHALL HAVE A CLEAR HEIGHT NOT EXCEEDING 30 TIMES THE WALL THICKNESS UNLESS SPECIFIED OTHERWISE BY THE CONTRACTOR'S SPECIALTY ENGINEER.</p> <p>6. NON-LOAD BEARING PARTITIONS SHALL NOT BE CONNECTED TO THE BASE BUILDING STRUCTURE IN A WAY THAT WOULD ALTER THE LATERAL STIFFNESS OF THE PRIMARY STRUCTURAL SYSTEM OR THE GRAVITY LOAD PATH.</p> <p>7. ANY CONNECTIONS OTHER THAN THOSE PROVIDING LATERAL SUPPORT OF THE WALL MUST BE SUBMITTED TO RJC FOR REVIEW AND COMMENT PRIOR TO COMMENCING CONSTRUCTION. THE SUBMISSION SHALL BE SIGNED AND SEALED BY THE SPECIALTY ENGINEER AND SHALL INDICATE ALL FACTORED FORCES IMPOSED ON THE PRIMARY STRUCTURAL SYSTEM.</p> <p>8. THE SPECIALTY ENGINEER SHALL ENSURE</p> <ul style="list-style-type: none">A. PROVIDE DEFLECTION GAPS BETWEEN NON-LOAD BEARING MASONRY AND ADJACENT COLUMNS AND WALLS EQUAL TO THE MAXIMUM SEISMIC DRIFT AT THAT LOCATION INDICATED ON THE "STRUCTURAL MOVEMENT" NOTES OR AS PROVIDED ON DETAILS, BUT IN NO CASE LESS THAN 1".B. MASONRY UNITS OF NON-LOAD BEARING WALLS BELOW OR ADJACENT TO GRAVITY STRUCTURE SHALL BE DESIGNED AND CONSTRUCTED TO ALLOW FOR THE FOLLOWING DEFLECTIONS:<ul style="list-style-type: none">i. FLOOR FRAMING - L/200 OR 1" (WHICHEVER IS GREATER)ii. ROOF FRAMING - L/180 OR 1" (WHICHEVER IS GREATER)C. DEFLECTION GAPS MAY BE FILLED WITH COMPRESSIBLE MATERIAL IF REQUIRED BUT THIS MATERIAL SHALL NOT IMPED THE OPENING OR CLOSING OF THE GAP DURING AN EARTHQUAKE. <p>9. FIELD REVIEW OF NON-LOAD BEARING MASONRY IS NOT PART OF RJC'S SCOPE AS REVIEW OF THE PRIMARY STRUCTURAL SYSTEM.</p> <p>10. THE SPECIALTY ENGINEER OR THEIR REPRESENTATIVE SHALL ENSURE QUALITY ASSURANCE OF THE WORK ON SITE BY PERFORMING ALL NECESSARY FIELD REVIEWS INCLUDING REVIEW OF THE PROVIDED DEFLECTION GAPS.</p> <p>11. UNLESS NOTED OTHERWISE, CONNECTION TO BASE BUILDING SHALL INCLUDE THE FOLLOWING:</p> <ul style="list-style-type: none">A. ALL VERTS TO HAVE MATCHING DOWELS FROM SLAB OR FOUNDATION BELOW.B. PROVIDE LATERAL RESTRAINT AT TOP PER DETAILS MW301, MW302 OR MW303 AS APPLICABLE, OR DETAILS PROVIDING EQUIVALENT OR BETTER RESTRAINT. IMPOSED RESTRAINT FORCES ON THE BASE BUILDING STRUCTURE TO BE INDICATED ON THE SHOP DRAWINGS.																				
<p>WOOD FRAMING - SHRINKAGE</p> <p>1. THE WOOD FRAME STRUCTURE WILL UNDERGO MOVEMENT DUE TO SHRINKAGE. SHRINKAGE OCCURS AS THE MOISTURE CONTENT IN WOOD DECREASES AS IT DRIES.</p> <p>2. KILN DRIED WOOD IS INITIALLY DRIED TO A MOISTURE CONTENT OF 19%. WOOD MAY TAKE ON MOISTURE DURING CONSTRUCTION DUE TO WEATHER.</p> <p>3. THE CONTRACTOR IS TO TAKE ALL REASONABLE MEASURES TO PROTECT WOOD FROM MOISTURE WHILE IT IS STORED ON SITE. THIS INCLUDES KEEPING WOOD STORED IN A DRY AREA OR PROTECTED WITH A MOISTURE RESISTANT COVER. THE CONTRACTOR SHOULD ALSO TAKE REASONABLE MEASURES TO CLOSE IN THE STRUCTURE AS QUICKLY AS POSSIBLE TO REDUCE THE EXPOSURE OF WOOD TO MOISTURE DURING CONSTRUCTION. BEAMS IN BALCONIES SHOULD ALSO BE COVERED AS LONG AS POSSIBLE.</p> <p>4. THE BUILDING WILL HAVE TWO STAGES OF CHANGE IN MOISTURE CONTENT IN THE WOOD. THE FIRST BEING THE CHANGE IN MOISTURE CONTENT UNTIL WHICH TIME THE STRUCTURE IS DRIED TO 19%. IT IS ASSUMED INITIALLY THE WOOD MAY BE AT 24% MOISTURE CONTENT AS AN UPPER LIMIT. THE SECOND BEING THE CHANGE FROM 19% MOISTURE CONTENT TO THE EQUILIBRIUM MOISTURE CONTENT OF 6% WHICH IS THE ANTICIPATED FINAL MOISTURE CONTENT EXPECTED DURING THE SERVICE OF THE BUILDING.</p> <p>5. THE TOTAL PER FLOOR SHRINKAGE TO BE TAKEN AS 1/2" BASED ON SHRINKAGE FROM 30% TO 6%.</p> <p>6. THE TOTAL PER FLOOR SHRINKAGE TO BE TAKEN AS 1/4" BASED ON SHRINKAGE FROM 19% TO 6%.</p> <p>7. FOR SYSTEMS OR COMPONENTS THAT ARE INSTALLED EARLY ON BEFORE THE BUILDING HAS BEEN DRIED TO 19%, ALLOW FOR THE FULL SHRINKAGE FROM 30% TO 6%. FOR SYSTEMS OR COMPONENTS THAT ARE ADDED AFTER THE BUILDING HAS BEEN DRIED TO 19%, ALLOW FOR THE SHRINKAGE BETWEEN 19% AND 6%.</p> <p>8. MOST OF THE SHRINKAGE OCCURS PERPENDICULAR TO THE GRAIN OF THE LUMBER OR CROSS GRAIN. THUS MOST OF THE SHRINKAGE OCCURS IN THE WALL PLATES AND FLOOR SYSTEM ITSELF WHERE THE WOOD IS STACKED IN PERPENDICULAR TO GRAIN LOADING. THE STRUCTURE SHRINKS VERY LITTLE OVER THE HEIGHT OF THE STUDS.</p> <p>9. FRAMING DETAILS SHALL ENSURE UNIFORM VERTICAL SHRINKAGE. ADJACENT PORTIONS OF STRUCTURE SHALL BE SUPPORTED ON ROUGHLY EQUIVALENT AMOUNTS OF HORIZONTAL TIMBER (JOISTS AND SLAT PLATES). DO NOT MIX KILNDRIED AND NON-KILNDRIED JOISTS IN ANY GIVEN FLOOR.</p> <p>10. FRAMING DETAILS AROUND NON-SHRINKING STRUCTURAL ELEMENTS (CONCRETE, STEEL, PARALLAMS, GLULAMS, MICROGLAMS, PLYWOOD ETC.) SHALL TAKE INTO ACCOUNT THE SHRINKAGE OF THE TIMBER. EXAMPLES:</p> <div></div> <p>11. THE TOTAL BUILDING SHORTENING DUE TO SHRINKAGE TO BE TAKEN AS FOLLOWS:</p> <table><tr><th>FLOOR LEVEL (i) ABOVE CONC. BASE STRUCTURE</th><th colspan="2">ESTIMATED SHRINKAGE AT FLOOR LEVEL (i)</th></tr><tr><th></th><th>19% MOISTURE CONTENT TO 6% MOISTURE CONTENT</th><th>30% MOISTURE CONTENT TO 6% MOISTURE CONTENT</th></tr><tr><td>8</td><td>1 1/16"</td><td>3 5/16"</td></tr><tr><td>4</td><td>1 5/16"</td><td>2 11/16"</td></tr><tr><td>3</td><td>1"</td><td>2"</td></tr><tr><td>2</td><td>1 1/16"</td><td>1 5/8"</td></tr><tr><td>1</td><td>5/16"</td><td>1 1/16"</td></tr></table>		FLOOR LEVEL (i) ABOVE CONC. BASE STRUCTURE	ESTIMATED SHRINKAGE AT FLOOR LEVEL (i)			19% MOISTURE CONTENT TO 6% MOISTURE CONTENT	30% MOISTURE CONTENT TO 6% MOISTURE CONTENT	8	1 1/16"	3 5/16"	4	1 5/16"	2 11/16"	3	1"	2"	2	1 1/16"	1 5/8"	1	5/16"	1 1/16"	<p>WOOD FRAMING - CONTRACTOR REQUIREMENTS</p> <p>1. PRIOR TO INSTALLING ANY WOOD FRAME, THE CONTRACTOR IS TO SURVEY THE SUPPORTING CONCRETE SLAB FOR LEVELNESS. WHERE THE FLOOR IS OUT OF LEVEL BY MORE THAN 1/2" FROM A HORIZONTAL DATUM BETWEEN ADJACENT HIGH POINTS, THE CONTRACTOR IS TO EITHER LEVEL THE SLAB WITH A SELF-LEVELING COMPOUND OR POUR LEVELING CURBS UNDER ALL BEARING WALLS.</p> <p>2. THE CONTRACTOR IS TO ENSURE THAT SHOP DRAWINGS FOR ALL SYSTEMS AND COMPONENTS THAT ARE DESIGNED BY OTHERS ARE PREPARED AHEAD AND SUBMITTED FOR REVIEW BY RJC AND THE ARCHITECT PRIOR TO INSTALLATION. THIS INCLUDES ENGINEERED WOOD SHOP DRAWINGS, ROOF ANCHORS, ROOF TOP MECHANICAL UNITS, OR OTHER ELEMENTS DESIGNED BY THE CONTRACTOR'S TRADES WHICH MAY AFFECT THE WOOD FRAME.</p> <p>3. THE CONTRACTOR IS TO COORDINATE A MEETING WITH THEIR TRADES AND RJC TO REVIEW THE ANTICIPATED BUILDING MOVEMENTS WHICH CAN OCCUR DURING CONSTRUCTION AND CONTINUE AFTER THE BUILDING IS IN SERVICE DUE TO WOOD SHRINKAGE. THE CONTRACTOR IS TO ENSURE THAT ALL TRADES ARE AWARE OF THE POTENTIAL BUILDING MOVEMENT AND HOW MUCH THEY NEED TO ACCOMMODATE FOR IN THEIR DETAILS AND CONNECTIONS. SEE SHRINKAGE NOTES.</p> <p>4. ALL FRAMING INCLUDING BACK FRAMING AND THE REMEDIATION OF ALL DEFICIENCIES IS TO BE SIGNED OFF BY RJC PRIOR TO INSTALLING ANY GYPCOTE OR CONCRETE TOPPING.</p> <p>5. THE CONTRACTOR IS TO ENGAGE THE BUILDING ENVELOPE CONSULTANT TO MONITOR THE GENERAL MOISTURE CONTENT OF THE WOOD STRUCTURE DURING CONSTRUCTION INCLUDING BOTH THE INTERIOR FRAMING AND EXTERIOR WALLS. THE WOOD STRUCTURE IS TO BE DRIED SUCH THAT THE MAXIMUM MOISTURE CONTENT IS BELOW 19% FOR ALL WOOD ELEMENTS PRIOR TO APPLYING FINISHES OR CLOSING IN ANY OF THE WOOD COMPONENTS. THE WOOD SHOULD BE LEFT TO DRY WITH AT LEAST ONE EXPOSED FACE UNTIL THE BUILDING ENVELOPE CONSULTANT HAS PROVIDED APPROVAL.</p>		<p>ARCHITECTURAL BRICK VENEER LEDGER ANGLES</p> <p>1. SEE AND COORDINATE WITH ARCHITECTURAL DRAWINGS FOR BRICK VENEERS. UNLESS NOTED OTHERWISE, 4" (90 mm) BRICK VENEERS SHALL BE SUPPORTED AT BOTTOM BY DIRECT BEARING ON CONCRETE SLAB OR CORBEL (IF SHOWN ON STRUCTURAL DRAWINGS) OR ON A STEEL LEDGER ANGLE.</p> <p>2. SEE ARCHITECTURAL DRAWINGS FOR BRICK SUPPORTS IN CASES DIFFERENT THAN SHOWN IN "ARCHITECTURAL BRICK VENEER LEDGER ANGLE" DETAILS.</p> <p>3. SEE ARCHITECTURAL DRAWINGS FOR LATERAL SUPPORT OF BRICK VENEER TO WALL BEHIND (CONCRETE, CONCRETE BLOCK, STEEL STUD, WOOD STUD, ETC.).</p> <p>4. CORROSION PROTECTION OF LEDGER ANGLES SHALL BE HOT-DIP GALVANIZING AFTER ALL SHOP FABRICATION, GRIND SMOOTH ALL WELDS. AFTER WELDING OR BOLTING THE ANGLE IN PLACE, COAT ALL NON-GALVANIZED STEEL (EXPOSED PORTION OF EMBED PLATES, WELDS, AND BOLT HEADS) WITH GALVACON OR EQUIVALENT ZINC RICH PAINT.</p> <p>5. PROVIDE LEDGER ANGLE AT EVERY FLOOR, UNLESS NOTED OTHERWISE.</p> <p>6. BRICK LEDGER ANGLES SHOULD BE INSTALLED WITH MINIMUM 6" CLEARANCE ABOVE SOIL AND 2" CLEARANCE ABOVE CONCRETE. SEE ARCHITECTURAL DRAWINGS FOR UNDERSIDE OF BRICK LEDGER ANGLE ELEVATIONS.</p> <p>7. SHOP DRAWINGS SHOWING ALL STEEL SUPPORTS FOR BRICK VENEER SHALL BE SUBMITTED FOR REVIEW. SHOP DRAWINGS SHALL INCLUDE ELEVATION DRAWINGS SHOWING DIMENSIONED LOCATIONS FOR ALL EMBEDS IN CONCRETE.</p> <p>8. U.N.O. BRICK LEDGER ANGLES SHOWN IN "ARCHITECTURAL BRICK VENEER LEDGER ANGLE" DETAILS MAY BE FLIPPED ON HORIZONTAL AXIS IF REQUIRED FOR ARCHITECTURAL ELEVATIONS.</p>	
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		<p>FIRE PROTECTION OF EXPOSED WOOD MEMBERS</p> <p>1. FIRE RATINGS PER ARCHITECTURAL DRAWINGS. ALL ELEMENTS DESIGNED FOR 1 HOUR RATING EXCEPT FOR ROOF.</p> <p>2. MASS WOOD ELEMENTS LISTED BELOW ARE EXPOSED AND ARE DESIGNED USING A RATIONAL DESIGN METHOD BASED ON ANNEX B OF CSA 086.</p> <ul style="list-style-type: none">A. THE UNDERSIDE OF ALL PANELS FOR BOTH FLOORS, AND ROOFS. <p>3. CLT FLOOR AND ROOF PANELS DESIGNED FOR THE RARE CASE FOR FIRE WITH 1.5 HRS DESIGN CASE (1.5" x 1.5" OR 1.0" x 1.0" x 1.5" IS MEMBER SECTIONS REDUCED BASED ON A NOMINAL CHAR RATE OF 1/32" (0.8 mm) / MINUTE + 1/32" (7 mm) FOR A HEAT AFFECTED ZONE. REDUCTIONS APPLY TO BOTTOM FACE ONLY. FACTORED RESISTANCES USING THE REDUCED DEPTH MODIFIED PER ANNEX B OF CSA 086: 1.5, 4.0, 6.0, 8.0, 10.0, 12.0, 15.0, 18.0, 20.0, 22.0, 24.0, 26.0, 28.0, 30.0, 32.0, 34.0, 36.0, 38.0, 40.0, 42.0, 44.0, 46.0, 48.0, 50.0, 52.0, 54.0, 56.0, 58.0, 60.0, 62.0, 64.0, 66.0, 68.0, 70.0, 72.0, 74.0, 76.0, 78.0, 80.0, 82.0, 84.0, 86.0, 88.0, 90.0, 92.0, 94.0, 96.0, 98.0, 100.0, 102.0, 104.0, 106.0, 108.0, 110.0, 112.0, 114.0, 116.0, 118.0, 120.0, 122.0, 124.0, 126.0, 128.0, 130.0, 132.0, 134.0, 136.0, 138.0, 140.0, 142.0, 144.0, 146.0, 148.0, 150.0, 152.0, 154.0, 156.0, 158.0, 160.0, 162.0, 164.0, 166.0, 168.0, 170.0, 172.0, 174.0, 176.0, 178.0, 180.0, 182.0, 184.0, 186.0, 188.0, 190.0, 192.0, 194.0, 196.0, 198.0, 200.0, 202.0, 204.0, 206.0, 208.0, 210.0, 212.0, 214.0, 216.0, 218.0, 220.0, 222.0, 224.0, 226.0, 228.0, 230.0, 232.0, 234.0, 236.0, 238.0, 240.0, 242.0, 244.0, 246.0, 248.0, 250.0, 252.0, 254.0, 256.0, 258.0, 260.0, 262.0, 264.0, 266.0, 268.0, 270.0, 272.0, 274.0, 276.0, 278.0, 280.0, 282.0, 284.0, 286.0, 288.0, 290.0, 292.0, 294.0, 296.0, 298.0, 300.0, 302.0, 304.0, 306.0, 308.0, 310.0, 312.0, 314.0, 316.0, 318.0, 320.0, 322.0, 324.0, 326.0, 328.0, 330.0, 332.0, 334.0, 336.0, 338.0, 340.0, 342.0, 344.0, 346.0, 348.0, 350.0, 352.0, 354.0, 356.0, 358.0, 360.0, 362.0, 364.0, 366.0, 368.0, 370.0, 372.0, 374.0, 376.0, 378.0, 380.0, 382.0, 384.0, 386.0, 388.0, 390.0, 392.0, 394.0, 396.0, 398.0, 400.0, 402.0, 404.0, 406.0, 408.0, 410.0, 412.0, 414.0, 416.0, 418.0, 420.0, 422.0, 424.0, 426.0, 428.0, 430.0, 432.0, 434.0, 436.0, 438.0, 440.0, 442.0, 444.0, 446.0, 448.0, 450.0, 452.0, 454.0, 456.0, 458.0, 460.0, 462.0, 464.0, 466.0, 468.0, 470.0, 472.0, 474.0, 476.0, 478.0, 480.0, 482.0, 484.0, 486.0, 488.0, 490.0, 492.0, 494.0, 496.0, 498.0, 500.0, 502.0, 504.0, 506.0, 508.0, 510.0, 512.0, 514.0, 516.0, 518.0, 520.0, 522.0, 524.0, 526.0, 528.0, 530.0, 532.0, 534.0, 536.0, 538.0, 540.0, 542.0, 544.0, 546.0, 548.0, 550.0, 552.0, 554.0, 556.0, 558.0, 560.0, 562.0, 564.0, 566.0, 568.0, 570.0, 572.0, 574.0, 576.0, 578.0, 580.0, 582.0, 584.0, 586.0, 588.0, 590.0, 592.0, 594.0, 596.0, 598.0, 600.0, 602.0, 604.0, 606.0, 608.0, 610.0, 612.0, 614.0, 616.0, 618.0, 620.0, 622.0, 624.0, 626.0, 628.0, 630.0, 632.0, 634.0, 636.0, 638.0, 640.0, 642.0, 644.0, 646.0, 648.0, 650.0, 652.0, 654.0, 656.0, 658.0, 660.0, 662.0, 664.0, 666.0, 668.0, 670.0, 672.0, 674.0, 676.0, 678.0, 680.0, 682.0, 684.0, 686.0, 688.0, 690.0, 692.0, 694.0, 696.0, 698.0, 700.0, 702.0, 704.0, 706.0, 708.0, 710.0, 712.0, 714.0, 716.0, 718.0, 720.0, 722.0, 724.0, 726.0, 728.0, 730.0, 732.0, 734.0, 736.0, 738.0, 740.0, 742.0, 744.0, 746.0, 748.0, 750.0, 752.0, 754.0, 756.0, 758.0, 760.0, 762.0, 764.0, 766.0, 768.0, 770.0, 772.0, 774.0, 776.0, 778.0, 780.0, 782.0, 784.0, 786.0, 788.0, 790.0, 792.0, 794.0, 796.0, 798.0, 800.0, 802.0, 804.0, 806.0, 808.0, 810.0, 812.0, 814.0, 816.0, 818.0, 820.0, 822.0, 824.0, 826.0, 828.0, 830.0, 832.0, 834.0, 836.0, 838.0, 840.0, 842.0, 844.0, 846.0, 848.0, 850.0, 852.0, 854.0, 856.0, 858.0, 860.0, 862.0, 864.0, 866.0, 868.0, 870.0, 872.0, 874.0, 876.0, 878.0, 880.0, 882.0, 884.0, 886.0, 888.0, 890.0, 892.0, 894.0, 896.0, 898.0, 900.0, 902.0, 904.0, 906.0, 908.0, 910.0, 912.0, 914.0, 916.0, 918.0, 920.0, 922.0, 924.0, 926.0, 928.0, 930.0, 932.0, 934.0, 936.0, 938.0, 940.0, 942.0, 944.0, 946.0, 948.0, 950.0, 952.0, 954.0, 956.0, 958.0, 960.0, 962.0, 964.0, 966.0, 968.0, 970.0, 972.0, 974.0, 976.0, 978.0, 980.0, 982.0, 984.0, 986.0, 988.0, 990.0, 992.0, 994.0, 996.0, 998.0, 1000.0.</p> <p>4. SEE ALSO SPECIFICATIONS AND DETAILS ON ARCHITECTURAL AND STRUCTURAL DRAWINGS FOR AREAS OF EXPOSED STEEL ELEMENTS WHERE INTUMESCENT PAINT REQUIRED.</p>		<p>WOOD FRAMING - MOISTURE BARRIERS</p> <p>1. PROVIDE A MOISTURE BARRIER BETWEEN WOOD ELEMENTS AND ALL CONCRETE OR MASONRY. IT CAN BE A SHEET OF LIGHT-GAUGE (3 GAUGE MINIMUM) GALVANIZED METAL, ASPHALT IMPREGNATED BUILDING PAPER (15 POUNDS PER 100 SQUARE FEET), CLOSED-CELL FOAM GASKET MATERIAL, TYPE 2 ROLL ROOFING, SHEET POLYETHYLENE NOT PERMITTED. ALL JUNCTIONS AND TERMINATIONS TO BE LAPPED 2" MINIMUM AND SEALED. BUTT JOINTS IN MOISTURE BARRIERS NOT PERMITTED.</p>																						

NOT FOR
CONSTRUCTION

Project Name

PROPOSED COLLEGE
DREAP APARTMENTS

1202 COLLEGE DRIVE
SASKATOON, SK

Sheet Title

GENERAL NOTES

Drawn By PML Scale As Indicated

Designed By KWM Date 2024.12.20

RJC Project Number SAS.138936.0001

Sheet Number Revision

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<p>WOOD FRAMING - WALL ANCHORAGE</p> <p>9. FASTEN LOAD BEARING WALLS AT BASE BY BOLTING THE BOTTOM PLATE (SILL PLATE) TO THE CONCRETE WITH 1/2" Ø ANCHORS AT 4'-0" O/C UNLESS NOTED OTHERWISE. ANCHORING OPTIONS ARE AS FOLLOWS:</p> <p>A. ANCHOR RODS CAST IN PLACE OR EPOXIED INTO PRE-DRILLED HOLES WITH THE HILTI HIT-4Y 200 (HIT-RE 50-V) ADHESIVE ANCHORING SYSTEM. ANCHOR RODS SHALL HAVE A MINIMUM 5" EMBEDMENT, MINIMUM 3" PROJECTION ABOVE THE CONCRETE, AND 2 1/2" OR GREATER EDGE DISTANCE.</p> <p>NOTE: ANCHOR RODS MUST BE HOOKED 1 1/2" AT THE BOTTOM IF THEY ARE NOT DEFORMED OR THREADED ALONG THEIR ENTIRE LENGTH.</p> <p>B. HILTI [Kwik Bolt] [Kwik Bolt TZ] MECHANICAL ANCHORS WITH A MINIMUM 3" EMBEDMENT, MINIMUM 3" PROJECTION ABOVE THE CONCRETE, AND 3" OR GREATER EDGE DISTANCE.</p> <p>C. SEE SHEAR WALL SCHEDULE AND TYPICAL DETAILS FOR ALL ANCHORING REQUIREMENTS OF SHEAR WALLS.</p> <p>10. NON-LOAD BEARING WALLS MAY BE FASTENED WITH 1/8" Ø POWER DRIVEN FASTENERS AT 16" O/C (MINIMUM 3/4" PENETRATION INTO CONCRETE).</p> <p>11. SILL PLATES SHALL BEAR ON A LEVEL SURFACE. PROVIDE A LEVELLING BED OF MORTAR IF REQUIRED. PROVIDE A SILL GASKET UNDER SILL PLATES BEARING ON CONCRETE. SEE NOTES ON "MOISTURE BARRIERS" FOR SILL GASKET REQUIREMENTS.</p>	<p>WOOD FRAMING - WALLS</p> <p>1. LOAD BEARING WALLS DENOTED ON PLAN THUS:</p> <p>ALL EXTERIOR WALLS ARE LOAD BEARING.</p> <p>2. SEE TYPICAL DETAILS FOR LOAD BEARING WALL CONNECTIONS BETWEEN FLOORS UNLESS NOTED OTHERWISE.</p> <p>3. UNLESS NOTED OTHERWISE, PROVIDE A BUILT-UP STUD POST AT THE ENDS OF ALL BEAMS AND GIRDER TRUSSES FRAMING INTO A WALL. THE BUILT-UP STUD POST SHALL MATCH THE WIDTH OF THE BEAM, AND THE STUD SIZE SHALL MATCH THOSE IN THE WALL U.N.O. ON PLAN.</p> <p>4. NAILING OF BUILT-UP STUD POSTS SHALL CONFORM TO THE FOLLOWING SCHEDULE. EACH STUD OF BUILT-UP POST SHALL BE NAILED.</p> <table><tr><th>STUD</th><th>NAILING</th></tr><tr><td>2 X 4</td><td>3" NAILS @ 8" O/C STAGGERED</td></tr><tr><td>2 X 6</td><td>2 ROWS OF 3" NAILS @ 8" O/C</td></tr><tr><td>2 X 8</td><td>2 ROWS OF 3" NAILS @ 8" O/C</td></tr></table> <p>BUILT-UP STUD POST NAILING PATTERNS (BY STUD SIZE):</p> <p>1 1/4" (1 1/2" FOR 4-PLY)</p> <p>BUILT-UP STUD POST NAILING PATTERNS (BY NUMBER OF PLYS):</p> <p>1/4" (8 mm) Ø ENGINEERED WOOD SCREWS (COUNTER-SUNK HEAD) AT 16" O/C. ALTERNATE SCREW DIRECTIONS</p> <p>5. ALL POSTS AND BUILT-UP STUD POSTS SHOWN ON ANY LEVEL SHALL BE CARRIED DOWN TO THE CONCRETE UNLESS NOTED OTHERWISE. PROVIDE SOLID BLOCKING BETWEEN JOISTS UNDER ALL POSTS AND BUILT-UP POSTS.</p> <p>6. ALL LOAD BEARING WALLS SHALL HAVE 2 CONTINUOUS TOP PLATES AND 1 CONTINUOUS BOTTOM PLATE. BEAMS OR HEADERS OVER OPENINGS IN WALLS SHALL BE DROPPED TO ALLOW THE TOP PLATES TO BE CONTINUOUS. WHERE 1 1/2" CYCLOTEC OR CONCRETE TOPPING IS USED ON THE FLOORS, PROVIDE 2 CONTINUOUS BOTTOM PLATES. DOUBLE PLATES SHALL BE SPICED WITH A MINIMUM 2'-0" STAGGER AND LAPPED AT CORNERS. TOP AND BOTTOM PLATES WHICH HAVE BEEN CORED OR WHICH ARE DISCONTINUOUS SHALL BE REINFORCED AS FOLLOWS:</p> <p>MAXIMUM 4" Ø HOLE IN 2 X 6 PLATE. MAXIMUM 2" Ø HOLE IN 2 X 4 PLATE. HOLE TO BE ON CENTERLINE OF PLATE.</p> <p>1 1/4" X 16" X 16 GAUGE STEEL STRAP EACH SIDE OF EACH PLATE. NAIL WITH 8-1/2" LONG NAILS AS SHOWN.</p> <p>1 1/4" X 16" X 16 GAUGE STEEL STRAP ON ONE SIDE OF EACH PLATE. NAIL WITH 8-1/2" LONG NAILS AS SHOWN.</p> <p>7. WHERE PERMANENT SHEATHING IS NOT APPLIED TO STUDS PROVIDE BLOCKING AT 3'-4" O/C FOR 2 X 4 WALLS AND 2'-0" O/C FOR 2 X 6 WALLS.</p> <p>8. ALL BEARING WALLS WITHOUT PLYWOOD SHEATHING BUT CLAD IN GYPSUM WALL BOARD TO HAVE STUD BLOCKING PER THE FOLLOWING TABLE. BLOCKING TO BE INSTALLED PRIOR TO PLACEMENT OF CYCLOTEC OR CONCRETE TOPPING.</p> <table><tr><th>STUD HEIGHT</th><th>BLOCKING REQUIREMENT</th></tr><tr><td>LESS THAN 4'-0"</td><td>NONE</td></tr><tr><td>4'-0" TO 8'-10"</td><td>1-ROW (MID HEIGHT)</td></tr><tr><td>8'-10" TO 11'-10"</td><td>2-ROWS (THIRD POINTS)</td></tr></table> <p>9. EXTERIOR WALL OPENINGS LESS THAN 1200 mm WIDE TO HAVE AT LEAST ONE SILL PLATE, AT TOP OF DOOR OPENINGS, AND AT TOP AND BOTTOM OF WINDOW OPENINGS. EXTERIOR WALL OPENINGS GREATER THAN 1200 mm WIDE TO HAVE AT LEAST TWO SILL PLATES, AT TOP OF DOOR OPENINGS, AND AT TOP AND BOTTOM OF WINDOW OPENINGS.</p> <p>10. NON-LOAD BEARING WALLS SHALL BE DETAILED TO ALLOW FOR DEFLECTION OF THE STRUCTURE TO PREVENT LOAD TRANSFER TO UNINTENDED ELEMENTS.</p> <p>11. WHERE A RIM BOARD IS NOT BRACED BY A FLOOR SYSTEM, SUCH AS AT A STAIR SHAFT, EITHER THE RIM BOARD SHALL BE FULL WIDTH OF THE WALL OR ALL WALL STUDS SHALL BE FULLY BLOCKED BESIDE THE RIM BOARD.</p>	STUD	NAILING	2 X 4	3" NAILS @ 8" O/C STAGGERED	2 X 6	2 ROWS OF 3" NAILS @ 8" O/C	2 X 8	2 ROWS OF 3" NAILS @ 8" O/C	STUD HEIGHT	BLOCKING REQUIREMENT	LESS THAN 4'-0"	NONE	4'-0" TO 8'-10"	1-ROW (MID HEIGHT)	8'-10" TO 11'-10"	2-ROWS (THIRD POINTS)	<p>ENGINEERED WOOD SCREWS</p> <p>1. ENGINEERED WOOD SCREWS SPECIFIED IN THE DRAWINGS ARE ASSY SCREWS AS MANUFACTURED BY SWG AND PROVIDED BY MTO SOLUTIONS.</p> <p>2. ALTERNATIVES WITH CANADIAN CONSTRUCTION MATERIALS CENTRE REPORTS (CMC) APPROVAL ARE TO BE PRE-APPROVED BY RJC.</p> <p>3. SCREW DESIGNATIONS:</p> <p>TYPE LENGTH (mm) DIAMETER (mm)</p> <p>4. SCREW TYPES ARE AS FOLLOWS:</p> <p>S1: SWG ASSY ECOFAST S2: SWG ASSY VG CSK S3: SWG ASSY SK S4: SWG ASSY VG CYL S5: SWG ASSY KOMB SDD: SELF-DRILLING DOWEL</p> <p>5. PROVIDE TAPERED HOLES IN STEEL SECTIONS TO SUIT FLUSH SCREW HEAD INSTALLATION.</p> <p>6. PRE-DRILL HOLES AS RECOMMENDED BY SCREW MANUFACTURER.</p> <p>7. SOME SAMPLE DETAILS ALSO REFERENCE USE OF SIMPSON STRONG TIE (SST) SDS SCREWS. ALTERNATIVES TO ALSO BE PRE-APPROVED BY RJC.</p> <p>ENGINEERED WOOD PRODUCTS (E.W.P.) GENERAL</p> <p>1. ENGINEERED WOOD PRODUCTS INCLUDE ALL PRE-MANUFACTURED BEAMS, COLUMNS, AND JOISTS BY WEYERHAEUSER TRUS JOIST AS SHOWN ON PLAN.</p> <p>2. BEAMS EXPOSED TO VIEW IN FINISHED BUILDING SHALL BE SANDED APPEARANCE GRADE WITH STAMPS IN COVERED LOCATIONS.</p> <p>3. ALL MANUFACTURED BEAMS, COLUMNS, AND JOISTS SHALL HAVE A MOISTURE CONTENT OF LESS THAN 12%. ALL WOOD SHALL BE WRAPPED AND PROTECTED FROM MOISTURE UNTIL IT IS INSTALLED.</p> <p>4. SIZES OF BEAMS AND POSTS SHALL BE AS SPECIFIED ON PLAN.</p> <p>5. BEAMS: MINIMUM STRENGTHS OF BEAMS AS SPECIFIED ON PLAN.</p> <table><tr><th>TRUS JOIST DESIGNATION</th><th>MODULUS OF ELASTICITY (E)</th><th>SHEAR RESISTANCE (V)</th><th>BENDING RESISTANCE (M)</th><th>BEARING RESISTANCE (P)</th></tr><tr><td>LSL</td><td>1,55E (1550 KSI)</td><td>575 PSI</td><td>4295 PSI</td><td>1535 PSI</td></tr><tr><td>LVL</td><td>2.2E (2000 KSI)</td><td>530 PSI</td><td>4805 PSI</td><td>1365 PSI</td></tr><tr><td>PBL</td><td>2.2E (2200 KSI)</td><td>540 PSI</td><td>5360 PSI</td><td>1135 PSI</td></tr></table> <p>BEAM DEFLECTIONS SHALL BE LIMITED TO SPAN/360 FOR LIVE LOAD AND SPAN/240 FOR TOTAL LOAD.</p> <p>LSL - LAMINATED STRAND LUMBER (TRUS JOIST TIMBERSTRAND) LVL - LAMINATED VENEER LUMBER (TRUS JOIST MICROLAM) PBL - PARALLEL STRAND LUMBER (TRUS JOIST PARALLAM)</p> <p>6. COLUMNS, COLUMNS SHALL BE PSL 1.8E BY WEYERHAEUSER TRUS JOIST OR PRE-APPROVED EQUIVALENT.</p> <p>7. UNLESS NOTED OTHERWISE ON PLAN STEEL CONNECTING HARDWARE FOR LSL, LVL, AND PBL BEAMS SHALL BE CAPABLE OF DEVELOPING 100% OF THE BEAM SHEAR CAPACITY.</p> <p>8. PRODUCT SUBSTITUTIONS MUST BE PRE-APPROVED.</p> <p>9. DO NOT SUBSTITUTE BUILT-UP MEMBERS OF SAWN TIMBER FOR ENGINEERED WOOD PRODUCTS.</p> <p>10. PSL USED IN EXTERIOR APPLICATION SHALL MEET THE EXPOSURE REQUIREMENTS SPECIFIED BY THE MANUFACTURER. DO NOT USE LSL OR LVL FOR EXTERIOR APPLICATIONS.</p> <p>11. ALL ENGINEERED WOOD PRODUCTS SHALL BE KEPT DRY AND PROTECTED FROM THE ENVIRONMENT DURING STORAGE ON OR OFF THE PROJECT SITE AS PER THE MANUFACTURER'S REQUIREMENTS. STORE MATERIAL ELEVATED FROM GROUND AND WRAPPED TO SHED MOISTURE.</p>	TRUS JOIST DESIGNATION	MODULUS OF ELASTICITY (E)	SHEAR RESISTANCE (V)	BENDING RESISTANCE (M)	BEARING RESISTANCE (P)	LSL	1,55E (1550 KSI)	575 PSI	4295 PSI	1535 PSI	LVL	2.2E (2000 KSI)	530 PSI	4805 PSI	1365 PSI	PBL	2.2E (2200 KSI)	540 PSI	5360 PSI	1135 PSI	<p>WOOD FRAMING - NAILING</p> <p>1. NAILING SHALL CONFORM TO THE BUILDING CODE PART 9, AND "WOOD BUILDING TECHNOLOGY" PUBLISHED BY THE CANADIAN WOOD COUNCIL. NAILING CALLED UP ON THESE DRAWINGS (E.G. FOR SHEATHING) IS BASED ON COMMON NAILS.</p> <p>2. UNLESS NOTED OTHERWISE NAIL ALL WALL, FLOOR, AND ROOF SHEATHING TO FRAMING MEMBERS WITH 12" WALLS, SPACE NAILS AT 6" O/C AT ALL SUPPORTED EDGES OF SHEATHING SHEETS AND AT 12" O/C AT ALL INTERMEDIATE SUPPORTS. FLOOR SHEATHING SHALL BE GLUED TO THE JOISTS IN ADDITION TO NAILING. SEE SHEAR WALL SCHEDULE OR DIAPHRAGM NAILING SCHEDULE FOR ADDITIONAL REQUIREMENTS.</p> <p>X" = 6" O/C (WALLS, FLOORS, AND ROOF) Y" = 12" O/C (WALLS, FLOORS, AND ROOF)</p> <p>SHEATHING NAILING PATTERN</p> <p>3. DO NOT OVERDRIVE NAILS. NAILS OVERDRIVEN BY MORE THAN 10% OF PANEL THICKNESS MAY BE REJECTED.</p> <p>4. ALL PNEUMATICALLY DRIVEN NAILS ARE TO HAVE FULL ROUND HEADS. PNEUMATIC NAILS THAT HAVE CLIPPED OR MOON SHAPED HEADS ARE NOT PERMITTED. MOST STRIP STYLE NAILERS USE CLIPPED OR MOON SHAPED HEADS. THEREFORE, ONLY COIL STYLE NAILERS THAT USE NAILS WITH FULL ROUND HEADS SHALL BE USED.</p> <p>PERMITTED NOT PERMITTED</p> <p>5. NAILS FOR JOIST HANGERS AND CONNECTION HARDWARE SHOULD BE AS SPECIFIED OR SUPPLIED BY THE MANUFACTURER.</p> <p>6. NAILS SHALL BE COMMON ROUND STEEL WIRE NAILS OR PNEUMATIC NAILS (P NAILS) WITH MINIMUM DIAMETERS PER THE FOLLOWING TABLE. NAILS ARE CALLED UP BY LENGTH AND SHALL CONFORM TO THE FOLLOWING TABLE.</p> <table><tr><th>LENGTH</th><th>PENNY WEIGHT</th><th>NAIL DIAMETER</th></tr><tr><td>2"</td><td>6d</td><td>0.113"</td></tr><tr><td>2 1/2"</td><td>8d</td><td>0.131"</td></tr><tr><td>3"</td><td>10d</td><td>0.148"</td></tr><tr><td>3 1/4"</td><td>12d</td><td>0.148"</td></tr><tr><td>3 1/2"</td><td>16d</td><td>0.162"</td></tr><tr><td>4"</td><td>20d</td><td>0.192"</td></tr><tr><td>4 1/2"</td><td>30d</td><td>0.207"</td></tr><tr><td>5"</td><td>40d</td><td>0.228"</td></tr></table> <p>NOTE: 3" X 0.131" NAILS MAY BE SUBSTITUTED FOR 3" X 0.148" NAILS PROVIDED ADDITIONAL NAILS ARE USED OR THE SPECIFIED NAIL SPACING IS REDUCED PER THE FOLLOWING TABLE.</p> <table><tr><th>3" X 0.148" NAIL SPACING SPECIFIED ON DRAWINGS</th><th>3" X 0.131" NAIL SPACING (20% MORE NAILS REQUIRED)</th></tr><tr><td>12"</td><td>10"</td></tr><tr><td>10"</td><td>8"</td></tr><tr><td>8"</td><td>6"</td></tr><tr><td>6"</td><td>4"</td></tr><tr><td>4"</td><td>3"</td></tr><tr><td>3"</td><td>2 1/2"</td></tr><tr><td>2"</td><td>NOT APPLICABLE</td></tr></table>	LENGTH	PENNY WEIGHT	NAIL DIAMETER	2"	6d	0.113"	2 1/2"	8d	0.131"	3"	10d	0.148"	3 1/4"	12d	0.148"	3 1/2"	16d	0.162"	4"	20d	0.192"	4 1/2"	30d	0.207"	5"	40d	0.228"	3" X 0.148" NAIL SPACING SPECIFIED ON DRAWINGS	3" X 0.131" NAIL SPACING (20% MORE NAILS REQUIRED)	12"	10"	10"	8"	8"	6"	6"	4"	4"	3"	3"	2 1/2"	2"	NOT APPLICABLE
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3" X 0.148" NAIL SPACING SPECIFIED ON DRAWINGS	3" X 0.131" NAIL SPACING (20% MORE NAILS REQUIRED)																																																																																	
12"	10"																																																																																	
10"	8"																																																																																	
8"	6"																																																																																	
6"	4"																																																																																	
4"	3"																																																																																	
3"	2 1/2"																																																																																	
2"	NOT APPLICABLE																																																																																	

1	ISSUED FOR 50% REVIEW	2024.12.20	KML
No.	Revision	Date	By

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Seal

**NOT FOR
CONSTRUCTION**


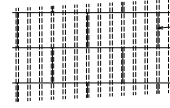
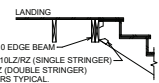
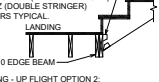
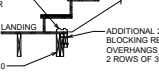
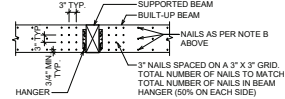

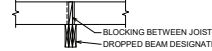
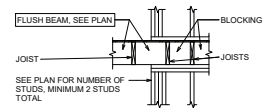
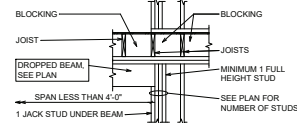
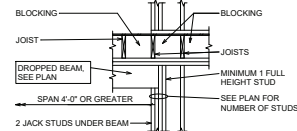
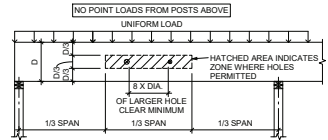
Project Name
**PROPOSED COLLEGE
DRIVE APARTMENTS**

1202 COLLEGE DRIVE
SASKATOON, SK

Sheet Title
GENERAL NOTES

Drawn By **KML** Scale **As Indicated**
Designed By **PWM** Date **2024.12.20**
RJC Project Number **SAS.138936.0001**
Sheet Number **1** Revision

S 1.9 **1**
2024-12-20 18:56:23

CROSS-LAMINATED TIMBER (CLT) 1. ALL DESIGN, DETAILS, MATERIALS AND CONSTRUCTION PROCEDURES SHALL CONFORM TO CURRENT EDITIONS OF: A. CANCSA-09S AND REFERENCED DOCUMENTS B. CSA B111 - WIRE NAILS, SPIKES AND STAPLES C. CLT HANDBOOK 2011 EDITION - FP INNOVATIONS D. PRG 320 E. ANNEX B OF CSA-C08 IF EUROPEAN PANELS ARE PROPOSED, THE SUPPLIER MUST SUPPLY AN EQUIVALENCY REPORT FROM AN INDEPENDENT TESTING AGENCY THAT MEET ALL THE REQUIREMENTS OF CSA AND PRG IN COMPARISON TO THE CANADIAN SPECIFICATION AND THAT TESTING MEETS THE REQUIREMENTS OF PRG 320. 2. CLT MEMBERS SHALL BE MANUFACTURED BY CSA APPROVED PLANT MEETING THE REQUIREMENTS OF ANSI/APA PRG 320. 3. CLT PANELS CONSIST OF DIMENSIONAL LUMBER, CROSSWISE STACKED AND GLUED TOGETHER, MINIMUM OF 3 LAYERS, MINIMUM THICKNESS TO BE 4 1/8" (102 mm). 4. WHERE CLT PANELS UTILIZED AT STAIR AND ELEVATOR CORES THEY ARE CONSIDERED BALLOON FRAMED TO MINIMIZE SHRINKAGE WITH LOADS, REACTIONS, AND PROPOSED CONNECTIONS INDICATED ON WALL ELEVATIONS, PLANS, AND SECTIONS TO FACILITATE PERFORMANCE BASED DESIGN BY SUPPLIER'S ENGINEER. 5. ANY CHANGES TO THE CLT FRAMING SHOWN ON THIS SET OF DRAWINGS SHALL HAVE PRIOR WRITTEN APPROVAL OF RJC. CHANGES WHICH HAVE NOT BEEN SO APPROVED WILL BE REJECTED. 6. CLT SIZES ON STRUCTURAL DRAWINGS MAY BE REVISED BY SUPPLIER IF SPECIALTY ENGINEER DESIGNS VARIANCES TO MEET ALL STRUCTURAL, ARCHITECTURAL, ACOUSTIC, AND FIRE REQUIREMENTS TO THE SATISFACTION OF RJC AND THE ARCHITECT. 7. CONFIRM ALL CLT DIMENSIONS AND OUTLINES WITH THE ARCHITECTURAL DRAWINGS. SEE ARCHITECTURAL DRAWINGS FOR ADDITIONAL DIMENSIONS, ELEVATIONS AND DETAILS. 8. THE SUPPLIER SHALL SUBMIT ADEQUATE PRODUCT CERTIFICATES TO RJC PRIOR TO COMMENCEMENT OF FABRICATION. 9. COORDINATE PENETRATIONS THROUGH CLT PANELS WITH ARCHITECTURAL, ELECTRICAL AND MECHANICAL DRAWINGS. ALL PENETRATIONS AND OPENINGS TO BE SHOWN ON SHOP DRAWINGS AND TO BE APPROVED BY RJC PRIOR TO FABRICATION. 10. AT JOINT LOCATIONS BETWEEN WALL TO FLOOR / ROOF PANELS AS WELL AS FLOOR / ROOF TO FLOOR / ROOF PANELS USE JOINT SEALANT FOR IMPROVED ACOUSTIC PERFORMANCE. SEE ARCHITECTURAL DRAWINGS FOR DETAILS. 11. ALL EXPOSED CLT CONNECTIONS SUPPORTING OR WITHIN FIRE RATED ASSEMBLIES TO MEET FIRE RESISTANCE RATING REQUIREMENTS. SEE ARCHITECTURAL DRAWINGS FOR DETAILS. 12. ALL EXPOSED SURFACES TO BE VISUAL GRADE S-P-F L3. SEE ARCHITECTURAL DRAWINGS FOR ADDITIONAL FINISH REQUIREMENTS. 13. ERECTOR / CONTRACTOR TO TAKE MEASURES TO ENSURE ALL CLT PANELS ARE KEPT DRY AND PROTECTED FROM THE ENVIRONMENT DURING STORAGE, TRANSPORTATION, AND HANDLING ON OR OFF THE PROJECT SITE TO MAINTAIN PROPER MOISTURE CONTENT. STORE MATERIAL ELEVATED FROM GROUND AND WRAPPED TO SHED MOISTURE. PROVIDE PRE-APPLIED MEMBRANE TO TOP AND SIDE SURFACES IN PLANT. WAX SEAL TO BE APPLIED TO CLT END GRAINS. 14. CLT MEMBERS SHALL BE KEPT DRY AND PROTECTED FROM WEATHER. A SHOP APPLIED WATERPROOFING ADHESIVE MEMBRANE OR OTHER APPROVED EQUIVALENT SHALL BE USED AS A MINIMUM. 15. CLT MOISTURE CONTENT 12% AVERAGE, WITH NO PIECE EXCEEDING 15%. 16. FLOOR VIBRATION TO ACHIEVE PEAK ACCELERATION THRESHOLD FOR RESIDENTIAL, AND OFFICE SPACE OF 0.05g. DUE TO WALKING EXCITATION AS DEFINED IN ASCE GUIDE 11. 17. ALTERNATE PRICE FOR 1LT FLOOR + PLYWOOD SHEATHING SUBSTITUTION (OR OTHER ONE WAY SYSTEM) HAS NOT BEEN STUDIED BY RJC AS PART OF THESE CURRENT DOCUMENTS AND IF PROVIDED AS PART OF THE BID AT THE SOLE DISCRETION OF THE SUPPLIER. PERFORMANCE OF THE ALTERNATE MUST BE EQUIVALENT OR BETTER THAN THE SPECIFIED SYSTEM. 18. SEE ARCHITECTURAL FOR FINISHING REQUIREMENTS. 19. SEE "FIRE PROTECTION OF EXPOSED WOOD MEMBERS" NOTE. 20. SEE "MASS TIMBER CONNECTIONS" NOTE. 21. REFER TO CLT SPECIFICATION FOR OTHER REQUIREMENTS. ELEVATORS - WOOD 1. SEE ELEVATOR SHOP DRAWINGS FOR ELEVATOR REQUIREMENTS, IN PARTICULAR: - CONFIRM SHAFT DIMENSIONS - CONFIRM PIT DEPTH - CONFIRM LOCATION OF DIVIDER BEAMS - CONFIRM ROUGH OPENING SIZE FOR DOORS 2. STEEL BEAMS PROVIDING PERMANENT SUPPORT FOR ELEVATOR MACHINERY AND EQUIPMENT ARE NOT SHOWN ON THESE DRAWINGS. THEY SHALL BE DESIGNED AND DETAIL BY THE ELEVATOR SUB-CONTRACTOR. SHOP DRAWINGS SHOWING THESE BEAMS SHALL BE REVIEWED BY RJC FOR THEIR EFFECT ON THE BASE BUILDING STRUCTURE. THE CONCRETE INFL. SLAB AT THE TOP OF THE ELEVATOR SHAFT IS NOT DESIGNED TO SUPPORT THE ELEVATOR MACHINERY AND EQUIPMENT. UNLESS NOTED OTHERWISE. IF THE ELEVATOR MANUFACTURER NEEDS THE SLAB TO SUPPORT THE ELEVATOR MACHINERY AND EQUIPMENT, THEN THE ELEVATOR MANUFACTURER SHALL ADVISE RJC AND SHALL PROVIDE LOAD INFORMATION AS REFERENCED IN NOTE 3. 3. THE ELEVATOR MANUFACTURER SHALL PROVIDE RJC WITH LOADING DATA IN ACCORDANCE WITH NATIONAL BUILDING CODE OF CANADA. REQUIREMENTS FOR ALL FORCES BEING IMPOSED ON THE STRUCTURE. ALL FORCES SHALL BE CLEARLY INDICATED IN THE REQUIRED LOCATIONS WITH APPLICABLE DIRECTIONS. ULTIMATE STRENGTH DESIGN FORCES SHALL BE PROVIDED. DEFLECTION DESIGN FORCES SHALL BE PROVIDED WITH ASSOCIATED DEFLECTION LIMITS, WHERE DEFLECTIONS ARE CRITICAL. 4. PROVIDE A W10x19 HOIST BEAM IN THE CEILING OF THE ELEVATOR MACHINE ROOM. SPANNING OVER THE ELEVATOR SHAFT. THE HOIST BEAM SHALL BE SUPPORTED ON EACH END AS PER WOOD ELEVATOR DETAILS ON S ____  <p>MAXIMUM SERVICE LOAD = 10 KIPS</p>	WOOD FRAMING - SHEATHING 1. A. <u>ROOF SHEATHING</u> FLAT ROOF (SLOPE 15% MAXIMUM) SIP TONGUE AND GROOVE PLYWOOD B. <u>EXTERIOR WALL SHEATHING</u> 3/8" OSB ON EXTERIOR SIDE TYPICAL. SEE ALSO ARCHITECTURAL FOR ADDITIONAL SHEATHING REQUIREMENTS. D. <u>SHEAR WALL SHEATHING</u> SEE SHEAR WALL SCHEDULE FOR SHEATHING REQUIREMENTS AT SHEAR WALL LOCATIONS. 2. LAY FLOOR AND ROOF SHEATHING WITH THE SURFACE GRAIN AT RIGHT ANGLES TO THE JOISTS. STAGGER THE JOINTS PARALLEL TO THE JOISTS.  <p>END JOINTS OF PANELS MUST BE SUPPORTED</p> 3. DRYWALL OR SHEATHING ON LOAD BEARING WALLS OR SHEAR WALLS SHALL BE FASTENED DIRECTLY TO THE STUDS, WITHOUT THE USE OF RESILIENT METAL CHANNELS. STAIR CONSTRUCTION 1. <u>TREADS</u> : STAIR TREADS SHALL CONFORM TO BUILDING CODE SECTION 9.8.9.5 AND SHALL BE CLOSED RISERS. 2. <u>STRINGERS</u> : STAIR STRINGERS SHALL BE SELECT STRUCTURAL D.FIR U.N.O. A. EXIT STAIRS WITH HALF HEIGHT LANDINGS: MAXIMUM 10'-0" FLOOR TO FLOOR, MAXIMUM 3'-8" WIDE. <ul style="list-style-type: none">STAIR STRINGERS SHALL BE SPACED AT MAXIMUM 2'-0" APART.INTERIOR STRINGERS SHALL BE MINIMUM 2 X 12 NOTCHEDEDGE STRINGERS SHALL BE MINIMUM 1-2 X 12 NOTCHEDNO OVER CUTTING OF STRINGERS ALLOWED.MINIMUM 1 1/2" THICK REQUIRED B. EXIT STAIRS WITHOUT HALF HEIGHT LANDINGS: MAXIMUM 10'-0" FLOOR TO FLOOR, MAXIMUM 3'-8" WIDE. <ul style="list-style-type: none">STAIR STRINGERS SHALL HAVE MIN. 2 X 12 EACH SIDE OF STAIRS. NO CENTER STRINGER.NO NOTCHING OF STRINGERS ALLOWED.TREADS SHALL BE MINIMUM 1 1/2" THICK AND SUPPORTED AT TOE AND HEEL OF TREAD BETWEEN STRINGERS. 3. TYPICAL LANDING - DOWN FLIGHT:  <p>MINIMUM 2-2 X 10 EDGE BEAM SIMPSON LSS210-2Z (DOUBLE STRINGER) SLOPED HANGERS TYPICAL</p> 4. TYPICAL LANDING - UP FLIGHT OPTION 1: SIMPSON SSS210-2Z (SINGLE STRINGER) AND LSS210-2Z (DOUBLE STRINGER) SLOPED HANGERS TYPICAL  <p>MINIMUM 2-2 X 10 EDGE BEAM</p> 5. TYPICAL LANDING - UP FLIGHT OPTION 2:  <p>MINIMUM 2-2 X 10 EDGE BEAM ADDITIONAL 2 X 6 CONTINUOUS BLOCKING REQUIRED IF STRINGER OVERHANGS EDGE BEAM C/W 2 ROWS OF 3" NAILS @ 8" O.C.</p>	WOOD FRAMING - BEAMS 1. BUILT-UP SAWN LUMBER BEAMS (E.G. 3-2 X 10) SHALL HAVE EACH PLY NAILED TOGETHER WITH COMMON NAILS AS FOLLOWS: A. FOR TOP LOADED BEAMS: 2 ROWS OF 3" NAILS AT 12" O.C. B. FOR BEAMS SIDE LOADED WITH UNIFORM JOISTS: <ul style="list-style-type: none">BEAM DEPTH < 8 1/4": 2 ROWS OF 3" NAILS AT 8" O.C.BEAM DEPTH > 8 1/4": 3 ROWS OF 3" NAILS AT 8" O.C. C. FOR BEAMS SIDE LOADED WITH A BEAM (POINT LOAD):  <p>3" TP SUPPORTED BEAM BUILT-UP BEAM NAILS AS PER NOTE B ABOVE 3" NAILS SPACED ON A 3" X 3" GRID TOTAL NUMBER OF NAILS TO MATCH TOTAL NUMBER OF NAILS IN BEAM HANGER (50% ON EACH SIDE)</p> <p>INDIVIDUAL MEMBERS MAY NOT BE SPLICED BETWEEN SUPPORTS.</p> 2. FOR ENGINEERED PRODUCTS, NAILING REQUIREMENTS OF LAMINATES SHALL BE SPECIFIED ON ENGINEERED SHOP DRAWINGS PROVIDED BY BEAM SUPPLIER. 3. FLUSH BEAMS:  <p>FLUSH BEAM TYPICAL U.N.O.</p> 4. DROPPED BEAMS:  <p>BLOCKING BETWEEN JOISTS DROPPED BEAM DESIGNATED AS (D.B.) ON PLAN</p> 5. UNLESS NOTED OTHERWISE ALL EXTERIOR WALL BEAMS, INTERIOR WALL BEAMS, AND DOOR HEADER BEAMS ARE DROPPED. UNLESS NOTED OTHERWISE ALL OTHER INTERIOR BEAMS ARE FLUSH. 6. USE 2 X 10 BEAMS OVER ALL OPENINGS IN BEARING WALLS UNLESS NOTED OTHERWISE. BEAMS SHALL BE SUPPORTED AT EACH END AS SHOWN BELOW UNLESS NOTED OTHERWISE. A. CASE 1 - FLUSH BEAM:  <p>FLUSH BEAM, SEE PLAN BLOCKING JOIST SEE PLAN FOR NUMBER OF STUDS, MINIMUM 2 STUDS TOTAL</p> B. CASE 2 - DROPPED BEAM - OPENING LESS THAN 4'-0":  <p>BLOCKING JOIST DROPPED BEAM, SEE PLAN MINIMUM 1 FULL HEIGHT STUD SPAN LESS THAN 4'-0" SEE PLAN FOR NUMBER OF STUDS</p> C. CASE 2 - DROPPED BEAM - OPENING 4'-0" OR GREATER:  <p>BLOCKING JOIST DROPPED BEAM, SEE PLAN MINIMUM 1 FULL HEIGHT STUD SPAN 4'-0" OR GREATER SEE PLAN FOR NUMBER OF STUDS 2 JACK STUDS UNDER BEAM</p> 7. HOLES IN BEAMS:  <p>NO POINT LOADS FROM POSTS ABOVE UNIFORM LOAD HATCHED AREA INDICATES ZONE WHERE HOLES PERMITTED 8 X DIA. OF LARGER HOLE CLEAR MINIMUM 1/3 SPAN 1/3 SPAN 1/3 SPAN</p> <table><tr><th>NOTES:</th><th>BEAM DEPTH</th><th>MAXIMUM HOLE DIAMETER</th></tr><tr><td>1. MAXIMUM 3 HOLES PER SPAN.</td><td>3 1/2"</td><td>3/8"</td></tr><tr><td>2. CONTACT RJC FOR OTHER CONDITIONS.</td><td>5 1/2"</td><td>1 1/8"</td></tr><tr><td></td><td>7 1/4"</td><td>1 1/2"</td></tr><tr><td></td><td>> 7 1/4"</td><td>2"</td></tr></table>	NOTES:	BEAM DEPTH	MAXIMUM HOLE DIAMETER	1. MAXIMUM 3 HOLES PER SPAN.	3 1/2"	3/8"	2. CONTACT RJC FOR OTHER CONDITIONS.	5 1/2"	1 1/8"		7 1/4"	1 1/2"		> 7 1/4"	2"
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No.	Revision	Date	By

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CONSTRUCTION**

Project Name
**PROPOSED COLLEGE
DRIVE APARTMENTS**

1202 COLLEGE DRIVE
SASKATOON, SK

Sheet Title
GENERAL NOTES

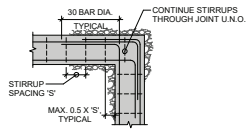
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Designed By **PWM** Date **2024.12.20**
RJC Project Number **SAS.138936.0001**
Sheet Number **1** Revision

S 1.10 **1**
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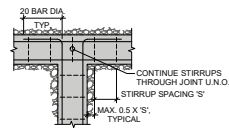


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rjc.ca
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tel. 306-868-2550

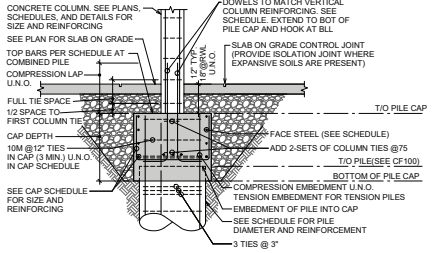
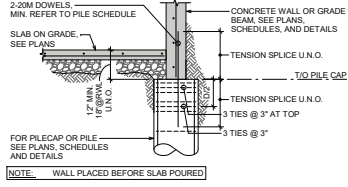
CF133
S 2.1
GRADE BEAM CORNER INTERSECTION
(PLAN VIEW)
1/2" = 1'-0"



CF130
S 2.1
GRADE BEAM TEE INTERSECTION
(PLAN VIEW)
1/2" = 1'-0"

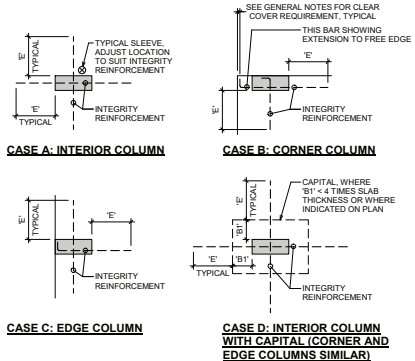


CF108
S 2.1
CONCRETE WALL / BEAM ON CONCRETE PILE
1/2" = 1'-0"



- NOTES:
1. COLUMN PLACED BEFORE SLAB POURED. IF SLAB ON GRADE IS POURED WITH INTEGRAL PEDESTAL TO TOP OF PILE CAP, LAP OF DOWELS TO COLUMN VERTICAL BARS TO BE MEASURED FROM TOP OF SLAB ON GRADE.
 2. DETAIL ONLY FOR CAPS CONCENTRICALLY LOCATED ON SINGLE PILES.
 3. FOR UNREINFORCED DRILLED PILES ADD DOWELS FROM PILE INTO CAP EQUIVALENT TO 1% OF PILE CROSS-SECTIONAL AREA (6-20M BARS MIN) WITH COMPRESSION DEVELOPMENT LENGTHS EACH END MIN.

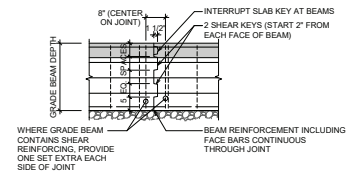
CF102
S 2.1
INTERIOR CONCRETE COLUMN ON PILE CAP
1/2" = 1'-0"



NOTE: DISTANCE 'E' SHALL BE OVERLAP WITH BOTTOM REINFORCEMENT OF TWO TIMES THE CASE 1 EMBEDMENT LENGTH OF THE INTEGRITY REINFORCEMENT BAR.

CS001
S 2.1
TYPICAL INTEGRITY REINFORCEMENT
CONFIGURATION, FLAT PLATES WITH
CONTINUOUS BOTTOM REINFORCEMENT
1/4" = 1'-0"

CF132
S 2.1
GRADE BEAM CONSTRUCTION JOINT
(ELEV VIEW)
1/2" = 1'-0"



NOTE: LOCATIONS OF CONSTRUCTION JOINTS TO BE PRE-APPROVED BY THE STRUCTURAL ENGINEER.

No.	Revision	Date	By
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Seal

NOT FOR CONSTRUCTION

Project Name
PROPOSED COLLEGE DRIVE APARTMENTS

1202 COLLEGE DRIVE
SASKATOON, SK

Sheet Title
TYPICAL DETAILS

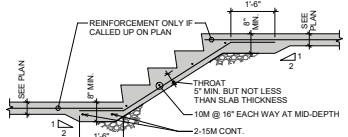
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Designed By **PWM** Date **2024.12.20**
RJC Project Number **SAS.138936.0001**

Sheet Number
S 2.1

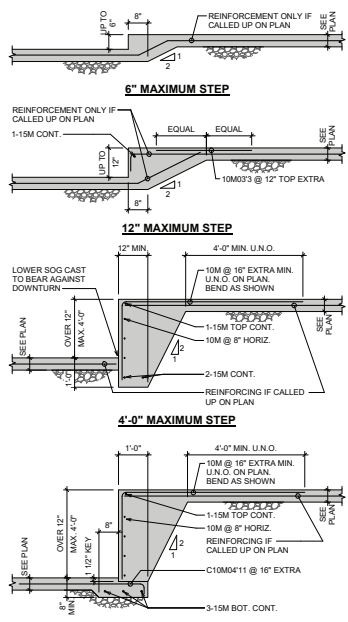
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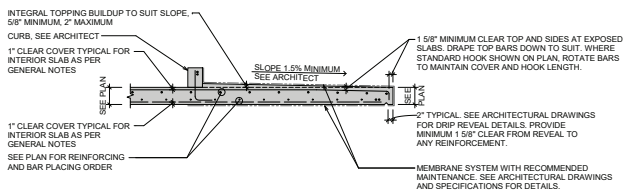
CG002
S 2.1
SLAB ON GRADE STAIRS
1/2" = 1'-0"



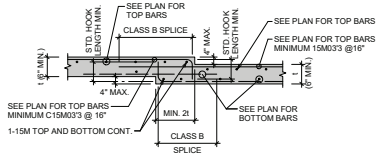
CG001
S 2.1
SLAB ON GRADE STEP DETAILS
1/2" = 1'-0"



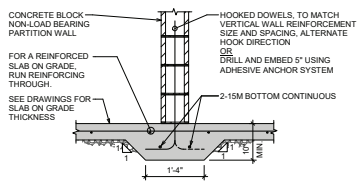
CS130
S 2.1
TYPICAL BALCONY OR EYEBROW WITH CURB SECTION
1/2" = 1'-0"

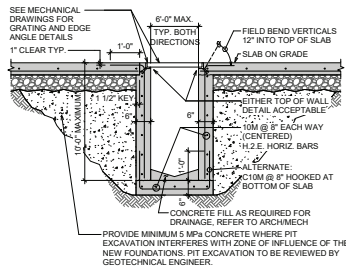


CS103
S 2.1
4" MAXIMUM SUSPENDED SLAB STEP
SOFFIT STEP
1/2" = 1'-0"



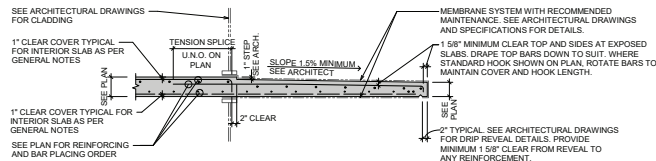
CG003
S 2.1
TYPICAL SLAB ON GRADE THICKENING
UNDER NON-LOAD BEARING BLOCK
PARTITION
3/4" = 1'-0"



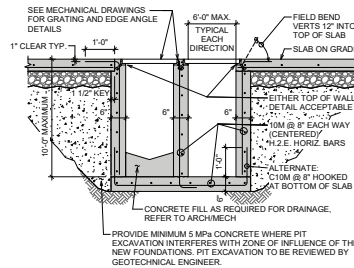


- NOTES:**
1. SEE MECHANICAL AND ARCHITECTURAL DRAWINGS AND SPECIFICATIONS FOR SIZE, LOCATION AND WATERPROOFING REQUIREMENTS.
 2. PRECAST ALTERNATIVE MAY BE PROPOSED BY CONTRACTOR IF ACCEPTABLE TO ARCHITECT AND MECHANICAL CONSULTANT. STRUCTURAL DESIGN OF PRECAST ALTERNATIVE NOT BY RJC. SUBMIT SHOP DRAWINGS AND RELATED SCHEDULES FOR PRECAST ALTERNATIVE SIGNED AND SEALED BY LICENSED SPECIALTY ENGINEER FOR REVIEW.

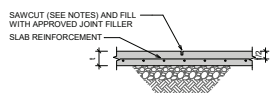
CF901 SUMP / ACCESS PIT
S 2.2 1/2" = 1'-0"



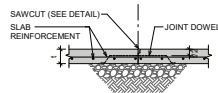
CS131 TYPICAL BALCONY OR EYEBROW WITH STEP DOWN SECTION
S 2.2 1/2" = 1'-0"



- NOTES:**
1. SEE MECHANICAL AND ARCHITECTURAL DRAWINGS AND SPECIFICATIONS FOR SIZE, LOCATION AND WATERPROOFING REQUIREMENTS.
 2. PRECAST ALTERNATIVE MAY BE PROPOSED BY CONTRACTOR IF ACCEPTABLE TO ARCHITECT AND MECHANICAL CONSULTANT. STRUCTURAL DESIGN OF PRECAST ALTERNATIVE NOT BY RJC. SUBMIT SHOP DRAWINGS AND RELATED SCHEDULES FOR PRECAST ALTERNATIVE SIGNED AND SEALED BY LICENSED SPECIALTY ENGINEER FOR REVIEW.



CG104 SINGLE LAYER OF REINFORCEMENT
S 2.2 1/2" = 1'-0"

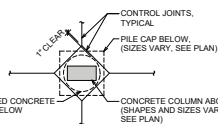


CG133 C: MEDIUM DUTY
S 2.2 1/2" = 1'-0"

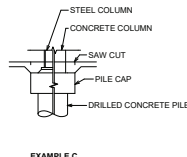
CF902 DOUBLE SUMP / ACCESS PIT
S 2.2 1/2" = 1'-0"

SAWCUT TABLE	
SLAB THICKNESS " (in)	REQUIRED SAWCUT DEPTH
1" - 5"	1/3 BUT NOT LESS THAN 1.5"
5" - 6"	
6" - 7"	
7" - 8"	1/3 BUT NOT LESS THAN 2"
8" - 10"	
10" - 12"	

CG106 SAWCUT TABLE
S 2.2 1:20

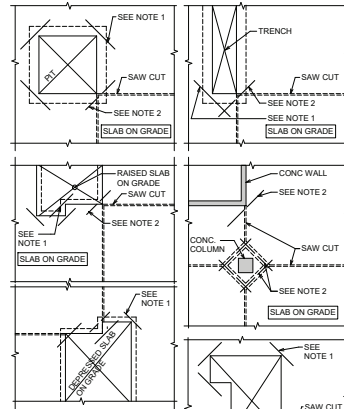


CG109 INTERIOR COLUMN ON CONCRETE PILES (WITH PILE CAP)
S 2.2 1/4" = 1'-0"



EXAMPLE C

ADDITIONAL SLAB BARS AT RE-ENTRANT CORNERS



- NOTES:**
1. 1-15M1500 T&B AT EACH CORNER TYP. U.N.O.
 2. WHERE CORNER TRIM BARS ARE SHOWN DASHED, THEY ARE NOT REQUIRED IF SAW CUTS ARE PROVIDED AS SHOWN, OTHERWISE PROVIDE 1-15M1500 T&B.
 3. FOLLOW DETAILS UNLESS NOTED OTHERWISE ON PLANS OR SECTIONS.
 4. ALTERNATE DETAIL FOR USE AT CONCRETE WALLS AND AT COLUMNS WHERE DIAMOND SHAPED JOINTS NOT ARCHITECTURALLY ACCEPTABLE.
- DISCONTINUOUS SAW CUT (AIM TO AVOID)
- FORM AND PROVIDE 13mm JOINT FILLER ALL SIDES
- 1-15M1500 T&B
- SAWCUT CONTROL JOINT
- INTERIOR CONCRETE COLUMN OR WALL

No.	Revision	Date	By
-----	----------	------	----

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Seal

**NOT FOR
CONSTRUCTION**

Project Name
**PROPOSED COLLEGE
DRIVE APARTMENTS**

1202 COLLEGE DRIVE
SASKATOON, SK

Sheet Title
TYPICAL DETAILS

Drawn By	KML	Scale	As Indicated
Designed By	PWM	Date	2024.12.20
RJC Project Number	SAS.138936.0001	Revision	
Sheet Number	S 2.2	Revision	1

2024-12-20 18:56:24

No.	Revision	Date	By
1	ISSUED FOR 50% REVIEW	2024.12.20	KML

Drawing Notes

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Seal

**NOT FOR
CONSTRUCTION**

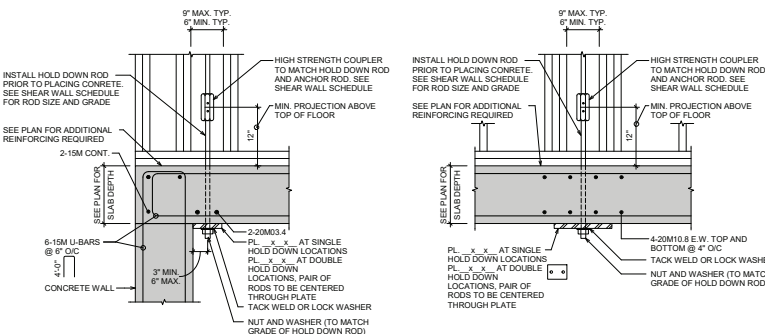
Project Name
**PROPOSED COLLEGE
DRIVE APARTMENTS**

1202 COLLEGE DRIVE
SASKATOON, SK

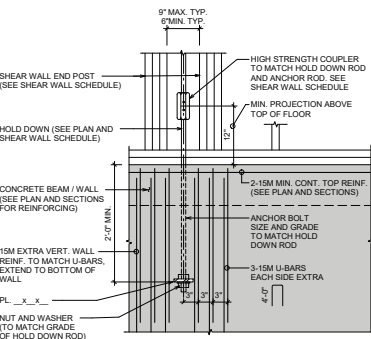
Sheet Title
TYPICAL DETAILS

Drawn By **KML** Scale **As Indicated**
Designed By **PWM** Date **2024.12.20**
RJC Project Number **SAS.138936.0001**
Sheet Number **S 2.3** Revision **1**

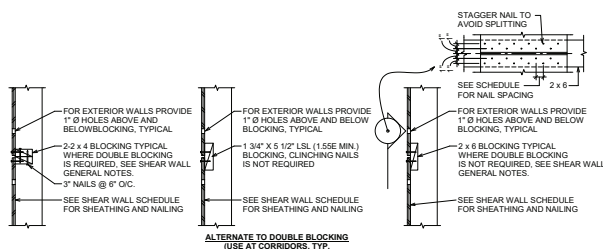
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WF004 HOLD DOWN CONNECTION TO CONCRETE SLAB
S2.3 1" = 1'-0"

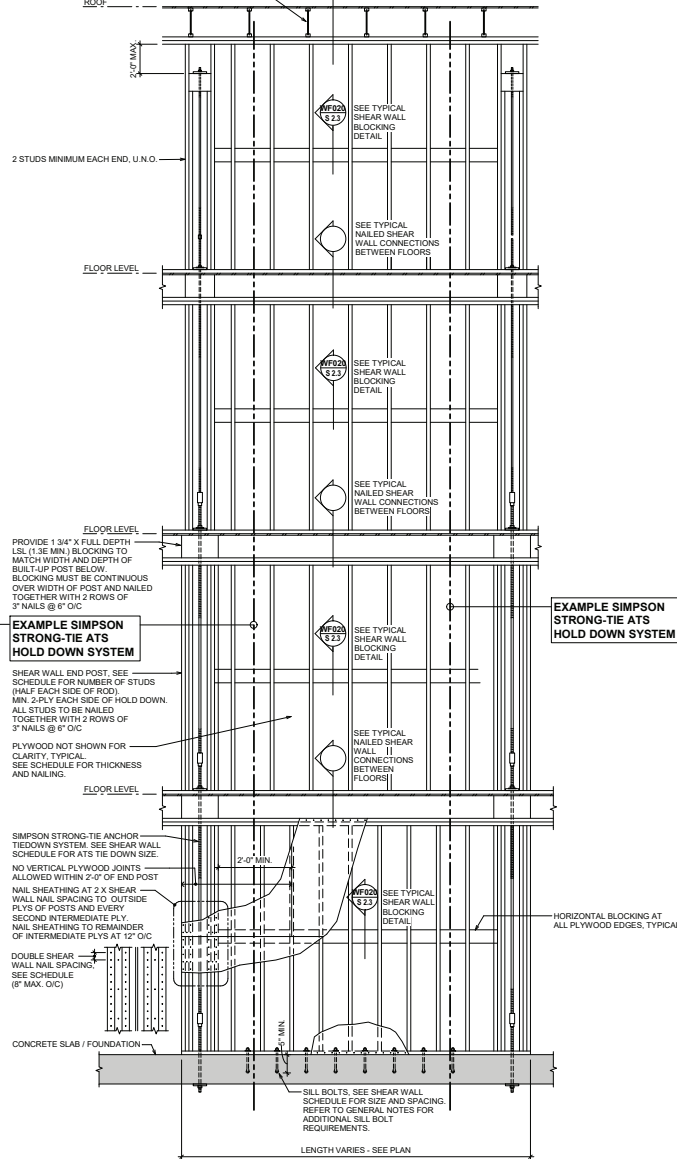


WF006 TYPICAL HOLD DOWN CONNECTION TO CONCRETE WALL
S2.3 1" = 1'-0"




WF020 TYPICAL SHEAR WALL BLOCKING DETAIL
S2.3 1" = 1'-0"

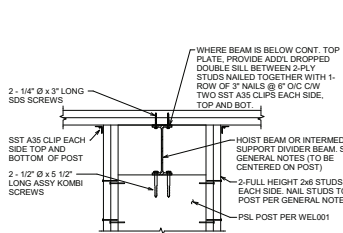
JOIST ROOF STRUCTURE TYPICAL DEPTH VARIES.
JOISTS MAY BE PARALLEL OR PERPENDICULAR TO
SHEAR WALL. SHEATHING TO CONTINUE TO US ROOF.



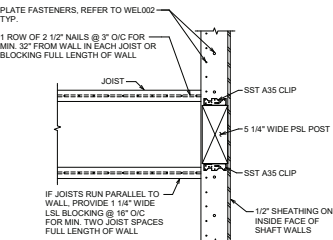
WF004 EXAMPLE 4 STOREY WOOD FRAME SHEAR WALL ELEVATION
S2.3 1/2\" = 1'-0"

WOOD FRAME SHEAR WALL SCHEDULE																		
NOTES	SW4	SW3	SW2	SW1	LEGEND	LEVEL												
1. SHEAR WALLS SHOWN ON PLAN AS THUS: SW TAG SHOWN ON PREFERRED SIDE OF WALL TO ATTACH SHEATHING  SHEAR WALL END POST (3 STUDS ON OUTSIDE, ROD, THEN REMAINDER OF STUDS ON THE INSIDE) 2. NAILS SHALL BE COMMON WIRE NAILS OR PNEUMATIC NAILS THAT ARE EQUIVALENT TO THE COMMON NAIL SIZES BELOW: <table><tr><th>LENGTH</th><th>DIAMETER</th><th>PENNY WEIGHT</th></tr><tr><td>2 1/2"</td><td>0.131"</td><td>64</td></tr><tr><td>3"</td><td>0.148"</td><td>104</td></tr><tr><td>3 1/2"</td><td>0.162"</td><td>164</td></tr></table> 3. ABBREVIATIONS USED IN THE SCHEDULE: B.S. ----- BOTH SIDES OF STUD S.S.T. ----- SIMPSON STRONG-TIE E.F.S. ----- EACH FACE STAGGERED T.O.W. ----- TOP OF WALL 4. NAILING SHOWN ON SCHEDULE APPLIES TO ALL FREE EDGES OF SHEATHING PANELS. PROVIDE NAILS AT 12" O.C. ALONG INTERMEDIATE SUPPORTS. 5. BLOCK ALL UNSUPPORTED EDGES WITH 2x BLOCKING. SEE GENERAL NOTES. TYPICAL SHEAR WALL BLOCKING DETAIL. DOUBLE STUDS AND DOUBLE BLOCKING ARE REQUIRED AT PANEL EDGES WHERE SHEAR WALL NAIL SPACING IS LESS THAN 7" O.C. OR NAIL LENGTH IS GREATER THAN 2 1/2" NAIL. DOUBLE STUDS / BLOCKING TOGETHER WITH TWO ROWS OF 3" NAILS AT 4" O.C. 6. DOUBLE RIM BOARD OR DOUBLE BLOCKING IS REQUIRED WHERE TWO ROWS OF SILL NAILS "B" ARE NOTED IN THE SHEAR WALL SCHEDULE. REFER TO TYPICAL DETAILS FOR ADDITIONAL REQUIREMENTS. 7. PROVIDE DOUBLE BOTTOM SILL PLATES FOR SHEAR WALLS SHEATHED BOTH SIDES OR FOR FLOORS WITH CONCRETE TOPPING. ALL STUDS IN SHEAR WALLS SHALL BE KILN DRIED D.FIR NO.1NO.2 GRADE OR BETTER.	LENGTH	DIAMETER	PENNY WEIGHT	2 1/2"	0.131"	64	3"	0.148"	104	3 1/2"	0.162"	164	1 ROW @ 4" O/C 16" O/C 7/16" OSB 1 SIDE 2 1/2" @ 4" O/C 4-2x4 MQCS-1 1/2 1 ROW @ 4" O/C 1 ROW @ 3" O/C	1 ROW @ 4" O/C 16" O/C 7/16" OSB 1 SIDE 2 1/2" @ 4" O/C 4-2x4 MQCS-1 1/2 1 ROW @ 4" O/C 1 ROW @ 3" O/C	1 ROW @ 6" O/C 24" O/C 7/16" OSB 1 SIDE 2 1/2" @ 4" O/C 4-2x4 MQCS-1 1/4 1 ROW @ 6" O/C 1 ROW @ 3" O/C	1 ROW @ 6" O/C 24" O/C 7/16" OSB 1 SIDE 2 1/2" @ 4" O/C 4-2x4 MQCS-1 1/4 1 ROW @ 6" O/C 1 ROW @ 3" O/C	NAILING TO BLOCKING / TRUSS	ROOF
LENGTH	DIAMETER	PENNY WEIGHT																
2 1/2"	0.131"	64																
3"	0.148"	104																
3 1/2"	0.162"	164																
	16" O/C 7/16" OSB 1 SIDE 2 1/2" @ 4" O/C 4-2x4 MQCS-1 1/2 1 ROW @ 4" O/C 1 ROW @ 3" O/C	16" O/C 7/16" OSB 1 SIDE 2 1/2" @ 4" O/C 4-2x4 MQCS-1 1/2 1 ROW @ 4" O/C 1 ROW @ 3" O/C	16" O/C 7/16" OSB 1 SIDE 2 1/2" @ 4" O/C 4-2x4 MQCS-1 1/4 1 ROW @ 4" O/C 1 ROW @ 3" O/C	16" O/C 7/16" OSB 1 SIDE 2 1/2" @ 4" O/C 4-2x4 MQCS-1 1/4 1 ROW @ 4" O/C 1 ROW @ 3" O/C	SHEAR CLIPS T.O.W. SHEATHING NAILING END POSTS HOLD DOWN (EACH END) SILL NAILS TYPE 'A' SILL NAILS TYPE 'B'	SIXTH FLOOR												
	16" O/C 7/16" OSB 1 SIDE 2 1/2" @ 4" O/C 4-2x4 MQCS-1 1/2 2 ROWS @ 3" O/C # 2 ROWS @ 5" O/C #	16" O/C 7/16" OSB 1 SIDE 2 1/2" @ 4" O/C 4-2x4 MQCS-1 1/2 2 ROWS @ 3" O/C # 2 ROWS @ 5" O/C #	16" O/C 7/16" OSB 1 SIDE 2 1/2" @ 4" O/C 4-2x4 MQCS-1 1/4 1 ROW @ 4" O/C 1 ROW @ 3" O/C	16" O/C 7/16" OSB 1 SIDE 2 1/2" @ 4" O/C 4-2x4 MQCS-1 1/4 1 ROW @ 4" O/C 1 ROW @ 3" O/C	SHEAR CLIPS T.O.W. SHEATHING NAILING END POSTS HOLD DOWN (EACH END) SILL NAILS TYPE 'A' SILL NAILS TYPE 'B'	FIFTH FLOOR												
	8" O/C 7/16" OSB 2 SIDES 2 1/2" @ 4" O/C 4-2x4 MQCS-1 1/2 2 ROWS @ 4" O/C # 2 ROWS @ 4" O/C #	8" O/C 7/16" OSB 2 SIDES 2 1/2" @ 4" O/C 4-2x4 MQCS-1 1/2 2 ROWS @ 4" O/C # 2 ROWS @ 4" O/C #	12" O/C 7/16" OSB 1 SIDE 2 1/2" @ 3" O/C 4-2x4 MQCS-1 1/4 1 ROW @ 3" O/C 1 ROW @ 3" O/C	12" O/C 7/16" OSB 1 SIDE 2 1/2" @ 3" O/C 4-2x4 MQCS-1 1/4 1 ROW @ 3" O/C 1 ROW @ 3" O/C	SHEAR CLIPS T.O.W. SHEATHING NAILING END POSTS HOLD DOWN (EACH END) SILL NAILS TYPE 'A' SILL NAILS TYPE 'B'	FOURTH FLOOR												
	12" O.C. 2 SIDES @ 7/16" OSB 2 SIDES 2 1/2" @ 3" O.C. † 10-2x4 MQCS-1 1/2 2 ROWS @ 3" O.C. # 2 ROWS @ 4" O.C. #	12" O.C. 2 SIDES @ 7/16" OSB 2 SIDES 2 1/2" @ 3" O.C. † 10-2x4 MQCS-1 1/2 2 ROWS @ 3" O.C. # 2 ROWS @ 4" O.C. #	10" O.C. 7/16" OSB 1 SIDE 2 1/2" @ 2" O.C. † 8-2x4 MQCS-1 1/4 1 ROW @ 3" O/C 1 ROW @ 3" O/C	10" O.C. 7/16" OSB 1 SIDE 2 1/2" @ 2" O.C. † 8-2x4 MQCS-1 1/4 1 ROW @ 3" O/C 1 ROW @ 3" O/C	SHEAR CLIPS T.O.W. SHEATHING NAILING END POSTS HOLD DOWN (EACH END) SILL NAILS TYPE 'A' SILL NAILS TYPE 'B'	THIRD FLOOR												
	12" O.C. 2 SIDES @ 7/16" OSB 2 SIDES 2 1/2" @ 3" O.C. † 10-2x4 MQCS-1 1/2 3/4" # @ 16" O/C	12" O.C. 2 SIDES @ 7/16" OSB 2 SIDES 2 1/2" @ 3" O.C. † 10-2x4 MQCS-1 1/2 3/4" # @ 16" O/C	10" O.C. 7/16" OSB 1 SIDE 2 1/2" @ 2" O.C. † 8-2x4 MQCS-1 1/4 3/4" # @ 16" O/C	10" O.C. 7/16" OSB 1 SIDE 2 1/2" @ 2" O.C. † 8-2x4 MQCS-1 1/4 3/4" # @ 16" O/C	SHEAR CLIPS T.O.W. SHEATHING NAILING END POSTS HOLD DOWN (EACH END) SILL BOLTS	SECOND FLOOR												

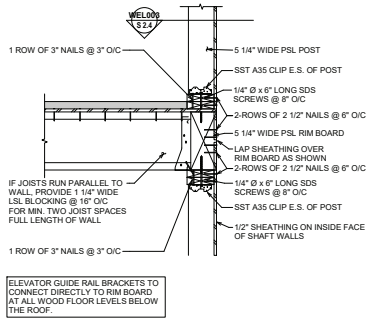
SYMBOL LEGEND:
† DOUBLE STUDS/DOUBLE BLOCKING (SEE NOTE 5)
DOUBLE RIM BOARD/DOUBLE BLOCKING (SEE NOTE 6)
◇ SHEAR WALL CLIPS ON BOTH SIDES OF WALL AT STATED SPACING



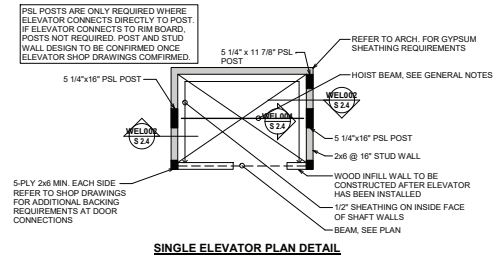
WEL004 HOIST BEAM TO PSL POST
S 24 1" = 1'-0"



WEL003 ELEVATOR WALL PLAN DETAIL
S 24 1" = 1'-0"



WEL002 JOISTS TO ELEVATOR WALL CONNECTION
S 24 1" = 1'-0"



NOTES:
1. REFER TO ELEVATOR SHOP DRAWINGS FOR LOCATION OF POSTS.
2. CONT. PSL POSTS ARE REQUIRED FROM LAST OCCUPIED LEVEL TO FORM ELEVATOR OVERRUN AND SUPPORT HOIST BEAM. TYPICAL 2 LOCATIONS (4 LOCATIONS FOR DOUBLE ELEVATOR). REFER TO WEL004.

WEL001 ELEVATOR PARTIAL PLAN
S 24 1/4" = 1'-0"

1	ISSUED FOR 50% REVIEW	2024.12.20	KML
No.	Revision	Date	By

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Scale

**NOT FOR
CONSTRUCTION**

Project Name
**PROPOSED COLLEGE
DRIVE APARTMENTS**

1202 COLLEGE DRIVE
SASKATOON, SK

Sheet Title
TYPICAL DETAILS

Drawn By **KML** Scale **As Indicated**
Designed By **PWM** Date **2024.12.20**
RJC Project Number **SAS.138936.0001**

Sheet Number
S 2.4
Revision
1

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Read Jones Christoffersen Ltd.
Engineers
rjc.ca

220 20th Street West, Suite 112
Saskatoon, SK S7M 0W9 Canada
tel 306-508-2550

COLUMN AND PILECAP LEGEND

INDICATES PILE OR PILECAP

COLUMN MARK
SEE COLUMN SCHEDULE

FACTORED LOAD/
UPLIFT LOAD/
FACTORED BENDING
MOMENT ON PILE

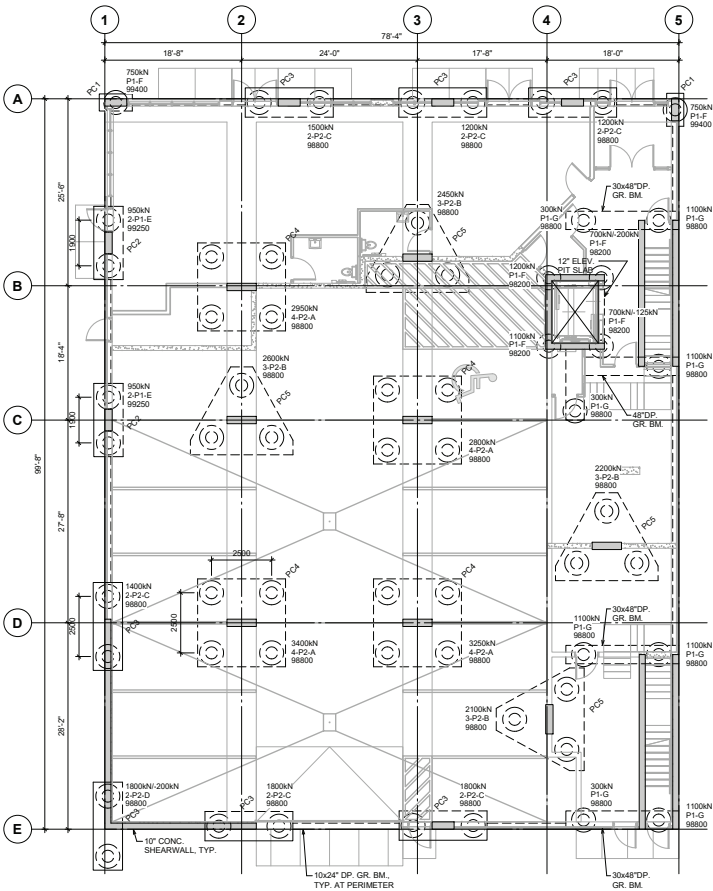
PILE TYPE
SEE SCHEDULE

PILECAP DETAIL, SEE DETAILS

ELEVATION (mm) TOP OF PILE
W/ REFERENCE TO MAIN
FLOOR ELEVATION = 10000 mm

PILE CAP SCHEDULE

MARK	SIZE
PC1	4'-6" X 2'-3" X 2'-0" DP
PC2	10'-0" X 4'-0" X 2'-6" DP
PC3	12'-0" X 4'-0" X 4'-0" DP
PC4	12'-0" X 12'-0" X 4'-0" DP
PC5	14'-0" X 12'-0" X 4'-0" DP



FOUNDATION PLAN
1/8" = 1'-0"

No.	Revision	Date	By
1	ISSUED FOR 50% REVIEW	2024.12.20	KML

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CONSTRUCTION**

Project Name
**PROPOSED COLLEGE
DRIVE APARTMENTS**

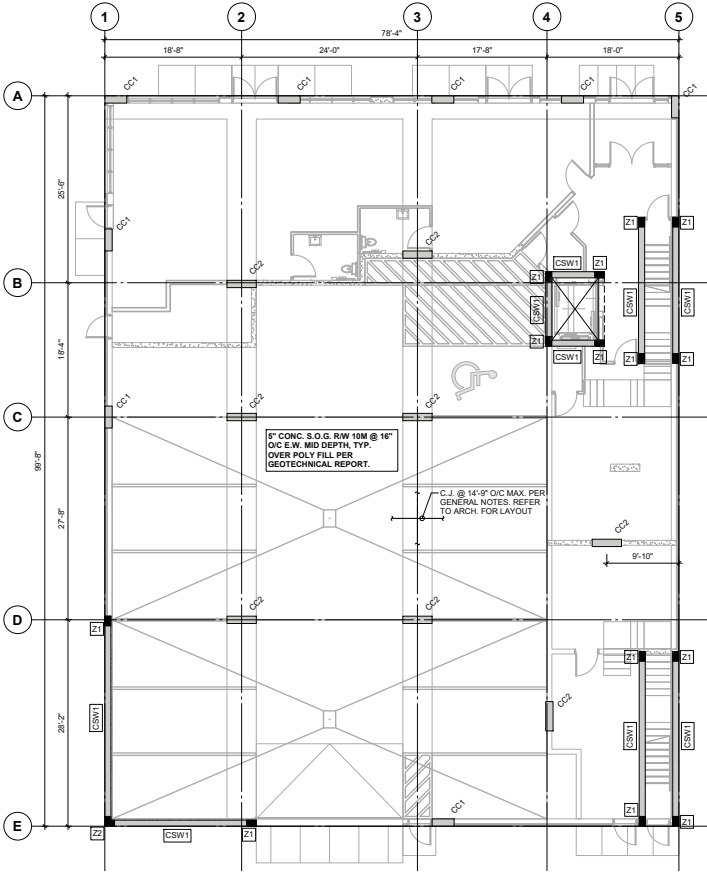
1202 COLLEGE DRIVE
SASKATOON, SK

Sheet Title
FOUNDATION PLAN

Drawn By	Author	Scale	As Indicated
Designed By	Designer	Date	2024.12.20
RJC Project Number	SAS.138936.0001		
Sheet Number	Revision		
S 3.1	1		

2024-12-20 18:56:25

CONCRETE COLUMN SCHEDULE			
MARK	SIZE	REINFORCING	
CC1	12" X 36"	10-20M VERTS, 10M TIES @ 12" O/C	
CC2	12" X 48"	10-25M VERTS, 10M TIES @ 12" O/C	
ZONE SCHEDULE			
	Z2	Z1	ELEMENT LEVEL
	8-15M VERT. 10M TIES @ 10" O/C	4-15M VERT. 10M TIES @ 10" O/C	LEVEL 1
CONC. SHEAR WALL SCHEDULE			
	CSW1	CONCRETE STRENGTH	ELEMENT LEVEL
	10" WALL R/W 10M @ 10" E.W. E.F.	25MPa UNCR.	LEVEL 1



 **MAIN FLOOR PLAN**
1/8" = 1'-0"



Read Jones Christoffersen Ltd.
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220 20th Street West, Suite 112
Saskatoon, SK S7M 0W9 Canada
tel: 306-938-2550

1	ISSUED FOR 50% REVIEW	2024.12.20	KML
No.	Revision	Date	By

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Seal

**NOT FOR
CONSTRUCTION**

Project Name
**PROPOSED COLLEGE
DRIVE APARTMENTS**

1202 COLLEGE DRIVE
SASKATOON, SK
Sheet Title
MAIN FLOOR PLAN

Drawn By	Author	Scale	As Indicated
Designed By	Designer	Date	2024.12.20
RJC Project Number	SAS.138936.0001		
Sheet Number	Revision		
S 3.2	1		

2024-12-20 18:56:25





SLAB STRUCTURE

1. BASIC BOTTOM REINFORCING MAT

18" DEEP SLAB R/W 20M @ 8" EACH WAY BOTTOM

PLUS EXTRA BOTTOM BARS AS SHOWN ON THE PLAN.
CENTER EXTRA BOTTOM BARS ALONG COLUMN CENTER LINES AND SPACE
AT 4" O/C UNLESS NOTED OTHERWISE ON THE PLAN. ALL BOTTOM
REINFORCING TO BE TENSION SPLICED

2. BAR PLACING ORDER:

4.  TOP UPPER LAYER (T.U.L.)
3.  TOP LOWER LAYER (T.L.L.)
2.  BOTTOM UPPER LAYER (B.U.L.)
1.  BOTTOM LOWER LAYER (B.L.L.)

CONCRETE OUTLINE LEGEND

← SLOPE SLAB OR SLAB BAND SOFFIT DOWN

 CONCRETE BUILD-UP USED TO FORM SLOPE. REFER TO ARCH.
SLAB DEPTHS NOTED ON PLAN DO NOT INCLUDE CONCRETE BUILD-UP.

● DENOTES MECHANICAL PENETRATION THROUGH SLAB.
SEE MECHANICAL DRAWINGS.

SHEARWALL HOLD-DOWN REINFORCING

 DENOTES SHEAR WALL HOLD-DOWN LOCATION

U.N.O. CENTER EXTRA REINFORCING AROUND HOLD-DOWN LOCATIONS, SPACE @ 4" O/C. U.N.O.

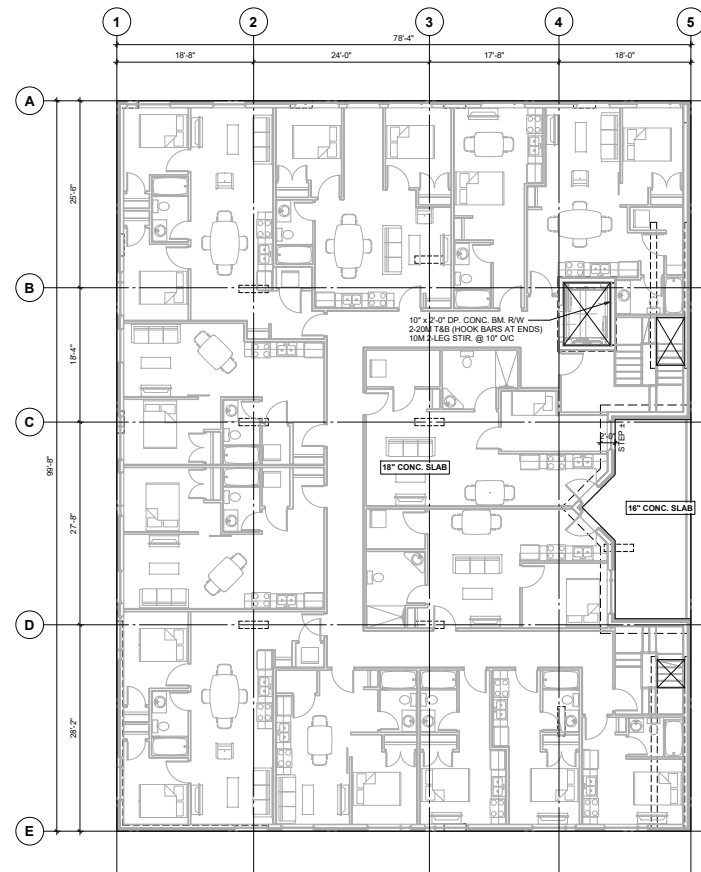
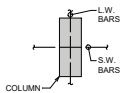
REFER TO TYPICAL SHEAR WALL HOLD-DOWN DETAILS FOR ADDITIONAL REQUIREMENTS.

CONTRACTOR TO LOCATE EXACT POSITION OF HOLD-DOWNS PRIOR TO PLACEMENT OF REINFORCING IN WALLS AND SLABS. SEE DETAILS / SECTIONS ON . IF DRILLING HOLES THROUGH CURED SLAB, ENSURE REINFORCING WILL NOT BE INTERFERING WITH THE HOLD-DOWN BOLT LOCATION.

INTEGRITY REINFORCING

1. UNLESS NOTED OTHERWISE ON PLAN, ALL FLAT PLATE SLABS TO HAVE THE FOLLOWING INTEGRITY REINFORCEMENT:

MARK	SLAB THICKNESS	REINFORCING
IR1	14"	3-25M E.W.
IR2	16"	4-25M S.W. + 3-35M L.W.



SECOND FLOOR PLAN
1/8" = 1'-0"

1	ISSUED FOR 50% REVIEW	2024.12.20	KML
No.	Revision	Date	By

Drawing Notes

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**NOT FOR
CONSTRUCTION**

Project Name

PROPOSED COLLEGE DRIVE APARTMENTS

1202 COLLEGE DRIVE
SASKATOON, SK

Sheet Title

SECOND FLOOR PLAN

Drawn By	Author	Scale	As indicated
Designed By	Designer	Date	2024.12.20

RJC Project Number **SAS.138936.0001**

Sheet Number Revision

S 3.3 **1**

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WOOD WALL LEGEND

DENOTES SHEAR WALL.

DENOTES LOAD BEARING WALL.

WOOD WALL SCHEDULE

FLOOR	EXTERIOR WALLS (2 x 6)	INTERIOR WALLS (2 x 4)	INTERIOR WALLS (2 x 6)	DOUBLE PARTY WALLS	STAGGERED STUD CORRIDOR WALLS
SIXTH TO ROOF	2 x 6 @ 16"	2 x 4 @ 16"	2 x 6 @ 16"	2 x 4 @ 16" (TWO WALLS)	2 x 4 @ 8"
FIFTH TO SIXTH	2 x 6 @ 16"	2 x 4 @ 12"	2 x 6 @ 16"	2 x 4 @ 16" (TWO WALLS)	2 x 4 @ 8"
FOURTH TO FIFTH	2 x 6 @ 16"	2 x 4 @ 12"	2 x 6 @ 16"	2 x 4 @ 16" (TWO WALLS)	2 x 4 @ 8"
THIRD TO FOURTH	2 x 6 @ 16"	2 x 4 @ 12"	2 x 6 @ 16"	2 x 4 @ 16" (TWO WALLS)	2 x 4 @ 8"
SECOND TO THIRD	2 x 6 @ 12"	2 x 4 @ 10"	2 x 6 @ 16"	2 x 4 @ 16" (TWO WALLS)	2 x 4 @ 8"

NOTES:

1. DENOTES DOUBLE RIM BOARD OR DOUBLE BLOCKING (S REQUIRED IN FLOOR LEVEL ABOVE WALL (I.E. MIN 2" 1 1/4" WIDE LSL RIM BOARD) (BLOCKING) REFER TO GENERAL NOTES AND TYPICAL DETAILS FOR ADDITIONAL REQUIREMENTS.

2. REFER TO "WOOD FRAMING" GENERAL NOTES AND WOOD SHEAR WALL SCHEDULE FOR ADDITIONAL REQUIREMENTS.

3. ALL STUDS TO BE S-P-F NO.1/NO.2 GRADE OR BETTER.

4. FOR STAGGERED STUD CORRIDOR WALLS:

WOOD FRAME SCHEDULE

WOOD JOIST SCHEDULE

DIMENSIONAL LUMBER JOISTS					
MARK	SIZE	TYPE	SPACING		
J1	2 x 6	SL	SEE PLAN		
J2	2 x 8	SL	SEE PLAN		
J3	2 x 10	SL	SEE PLAN		
J4	2 x 12	SL	SEE PLAN		

WOOD BEAM SCHEDULE

MARK	SIZE	TYPE	MARK	SIZE	TYPE
B1	2 x 6	SL	B6	1 3/4" x XXX XX"	LSL 1.55E
B2	2 x 8	SL	B7	1 3/4" x XXX XX"	LVL 2.0E
B3	2 x 10	SL	B8	3 1/2" x XXX XX"	PSL 2.2E
B4	2 x 12	SL	B9	5 1/4" x XXX XX"	PSL 2.2E
B5	1 3/4" x 9 1/4"	LVL 2.0E	B10	7" x XX XX"	PSL 2.2E

WOOD POST SCHEDULE

MARK	SIZE	TYPE	MARK	SIZE	TYPE
P1	2 x 4	SL	P6		
P2	2 x 6	SL			
P3	4 x 4	SL			
P4	6 x 6	SL			

NOTES:

1. FLOOR AND ROOF FRAMING SHOWN ON THIS PLAN IS FOR THE LEVEL ABOVE. DOOR AND WINDOW HEADERS SHOWN ARE OVER THE DOOR AND WINDOW AT THIS LEVEL.

2. SEE PLAN FOR NUMBER OF LAMINATIONS REQUIRED. EXAMPLE: 3B1 = 3 x 2x6 MEMBERS

3. PROVIDE NUMBER OF JACK STUDS PER GENERAL NOTES UNLESS NOTED OTHERWISE ON PLAN. WHERE ADDITIONAL JACK STUDS ARE REQUIRED THE FOLLOWING CONVENTION WILL BE USED. 4B1/3/4 DENOTES 4 STUDS TOTAL, 3 OF WHICH ARE JACK STUDS.

4. ABBREVIATIONS: SL -----SAWN LUMBER LSL ----- LAMINATED STRAND LUMBER PSL -----PARALLEL STRAND LUMBER LVL ----- LAMINATED VENEER LUMBER

5. I-JOIST HANGERS TO HAVE A MINIMUM CAPACITY OF V1 = 7.5 kN U.N.O. ON PLAN.

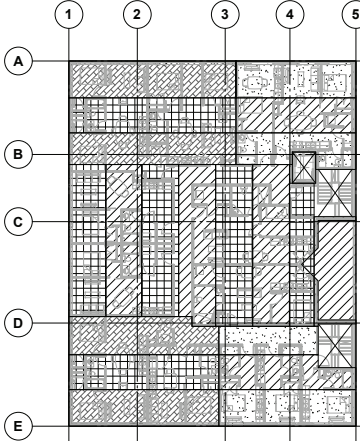
6. UNLESS NOTED OTHERWISE, BEAM HANGERS TO BE AS FOLLOWS. ALTERNATE HANGERS TO BE PRE-APPROVED BY RJC AND MUST PROVIDE EQUIVALENT OR GREATER CAPACITY.

1 3/4" WIDE MEMBER - SIMPSON STRONG TIE HUB V1 = 25 kN (5,685 LBS)

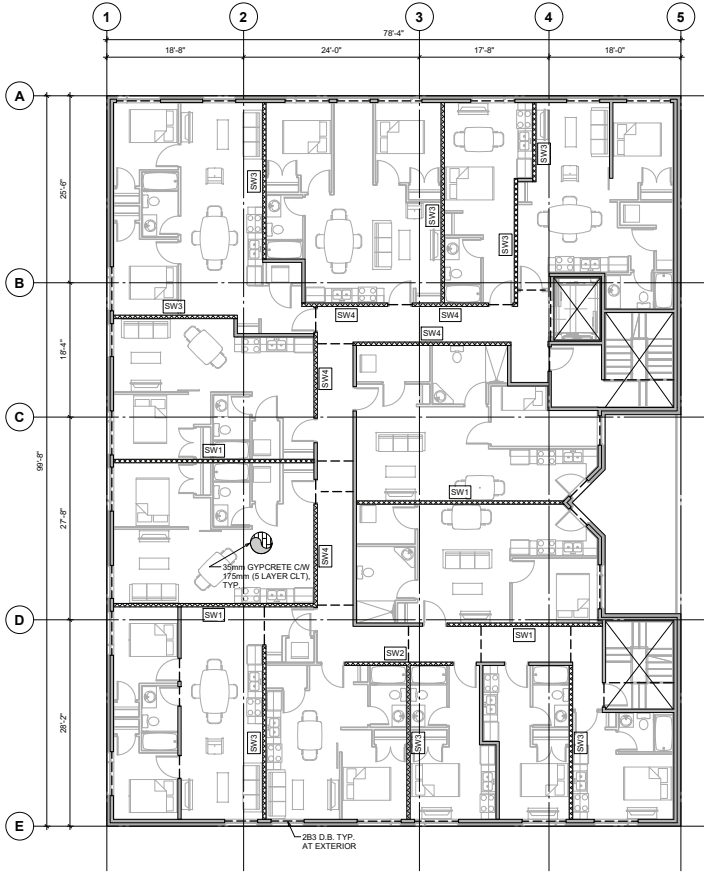
3 1/2" WIDE MEMBER - SIMPSON STRONG TIE HGUSA10 V1 = 62 kN (14,015 LBS)

5 1/4" WIDE MEMBER - SIMPSON STRONG TIE HGUSA10 V1 = 62 kN (14,045 LBS)

7" WIDE MEMBER - SIMPSON STRONG TIE HGUST25/10 V1 = 70 kN (15,760 LBS)



CLT PANEL LAYOUT
1/16" = 1'-0"



SECOND FLOOR PLAN SHOWING THIRD FLOOR FRAMING OVER
1/8" = 1'-0"

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**NOT FOR
CONSTRUCTION**

Project Name
**PROPOSED COLLEGE
DRIVE APARTMENTS**

1202 COLLEGE DRIVE
SASKATOON, SK

Sheet Title
**SECOND FLOOR PLAN
SHOWING THIRD FLOOR
FRAMING OVER**

Drawn By	Author	Scale	As Indicated
Designed By	Designer	Date	2024.12.20
RJC Project Number	SAS.138936.0001	Revision	

Sheet Number
S 3.4

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No.	Revision	Date	By
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Project Name

**PROPOSED COLLEGE
DRIVE APARTMENTS**

**1202 COLLEGE DRIVE
SASKATOON, SK**

Sheet Title

**THIRD FLOOR PLAN
SHOWING FOURTH FLOOR
FRAMING OVER**

Drawn By	Author	Scale	As Indicated
Designed By	Designer	Date	2024.12.20
RJC Project Number	SAS.138936.0001		

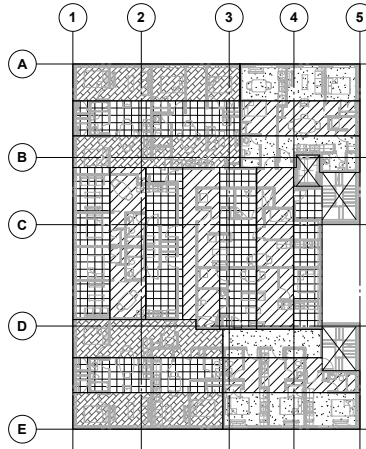
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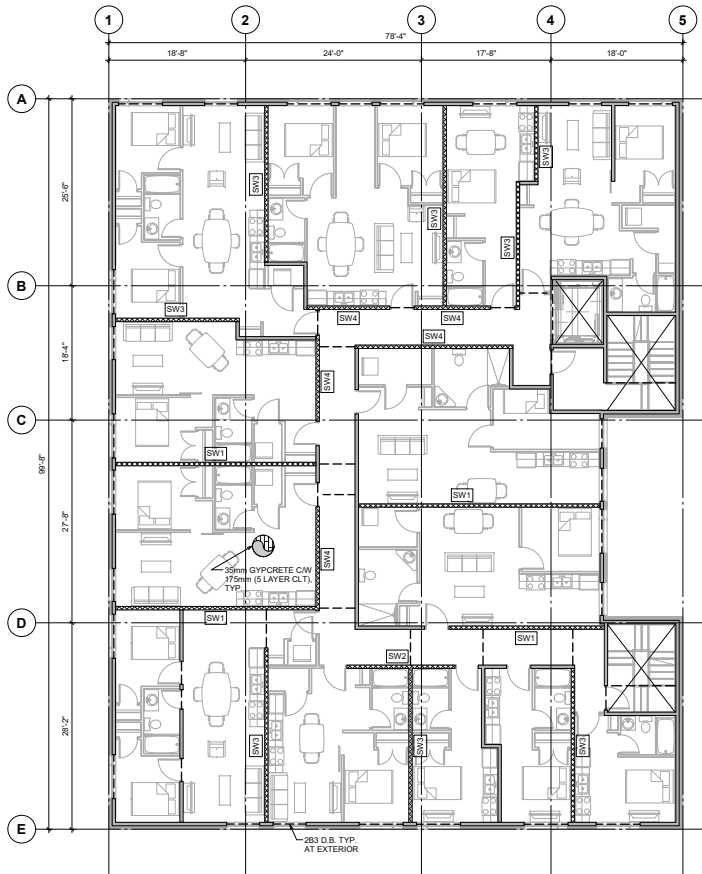
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WOOD WALL LEGEND

- DENOTES SHEAR WALL.
- DENOTES LOAD BEARING WALL.



CLT PANEL LAYOUT
1/16" = 1'-0"



THIRD FLOOR PLAN SHOWING FOURTH FLOOR FRAMING OVER
1/8" = 1'-0"



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Project Name

**PROPOSED COLLEGE
DRIVE APARTMENTS**

**1202 COLLEGE DRIVE
SASKATOON, SK**

Sheet Title

**FOURTH FLOOR PLAN
SHOWING ROOF FRAMING
OVER**

Drawn By	Author	Scale	As Indicated
Designed By	Designer	Date	2024.12.20
RJC Project Number	SAS.138936.0001		
Sheet Number	Revision		

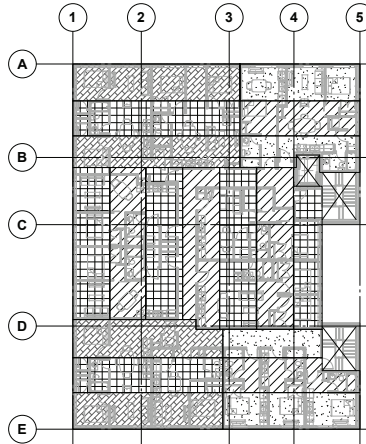
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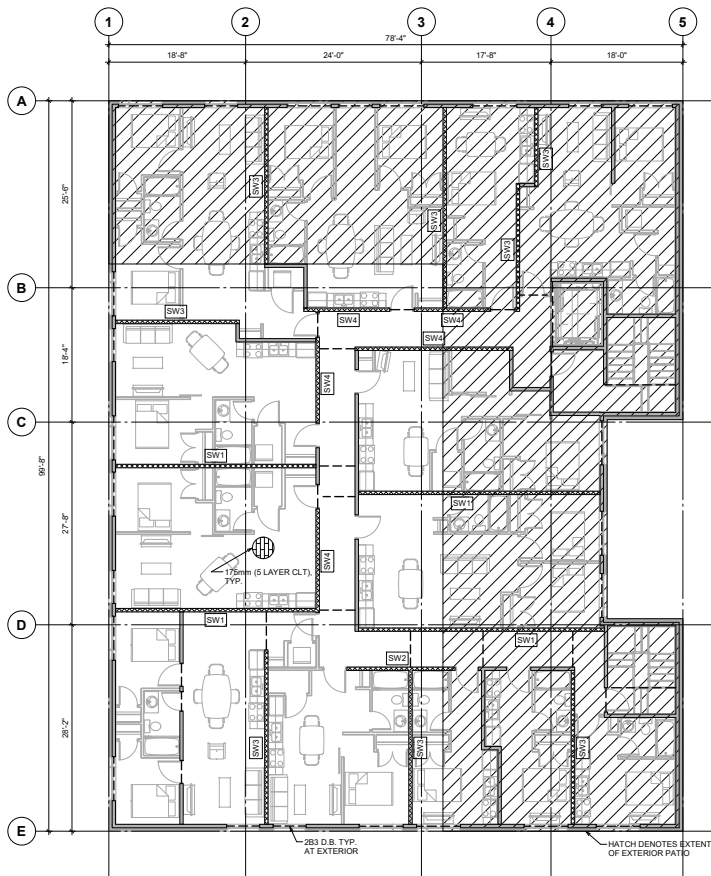
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WOOD WALL LEGEND

- DENOTES SHEAR WALL.
- DENOTES LOAD BEARING WALL.



CLT PANEL LAYOUT
1/16" = 1'-0"



FOURTH FLOOR PLAN SHOWING ROOF FRAMING OVER
1/8" = 1'-0"